



Cite this: DOI: 10.1039/c9ew00384c

Screen versus cyclone for improved capacity and robustness for sidestream and mainstream deammonification†

Tim Van Winckel,^{abc} Siegfried E. Vlaeminck,^{id}*^{ad} Ahmed Al-Omari,^b Benjamin Bachmann,^e Belinda Sturm,^{id}^c Bernhard Wett,^f Imre Takács,^g Charles Bott,^h Sudhir N. Murthyⁱ and Haydée De Clippeleir^b

Deammonification systems are being implemented as cost- and resource-efficient nitrogen removal processes. However, their complexity is a major hurdle towards successful transposition from side- to mainstream application. Merely out-selecting nitrite oxidizing bacteria (NOB) or retaining anammox bacteria (AnAOB) does not guarantee efficient mainstream deammonification. This paper presents for the first time the interactions and synergies between kinetic selection, through management of residual substrates, and physical selection, through separation of solid retention times (SRTs). This allowed the formulation of tangible operational recommendations for successful deammonification. Activity measurements were used to establish retention efficiencies (η) for AnAOB for full-scale cyclones and rotating drum screens installed at a sidestream and mainstream deammonification reactor (Strass, Austria). In the sidestream reactor, using a screen (η = 91%) instead of a cyclone (η = 88%) may increase the capacity by up to 29%. For the mainstream reactor, higher AnAOB retention efficiencies achieved by the screen (η = 72%) compared to the cyclone (η = 42%) induced a prospective increase in capacity by 80–90%. In addition, the switch in combination with bioaugmentation from the sidestream made the process less dependent on nitrite availability, thus aiding in the outselection of NOB. This allowed for a more flexible (intermittent) aeration strategy and a reduced need for tight SRT control for NOB washout. A sensitivity analysis explored expected trends to provide possible operational windows for further calibration. In essence, characterization of the physical selectors at full scale allowed a deeper understanding of operational windows of the process and quantification of capacity, ultimately leading to a more space and energy conservation process.

Received 7th May 2019,
Accepted 17th August 2019

DOI: 10.1039/c9ew00384c

rscl.li/es-water

Water impact

Deammonification is a sustainable alternative to conventional biological nutrient removal. We present a concept combining metabolic and physical selection for deammonification systems to manage the activity and retention of the different microbial species. This approach determined that switching from cyclones to screens (physical selection), which improved anammox retention, increased the capacity and decreased the process control (metabolic selection) for mainstream deammonification applications.

^a Center of Microbial Ecology and Technology (CMET), Faculty of Bioscience Engineering, Ghent University, Gent, Belgium

^b District of Columbia Water and Sewer Authority, Blue Plains Advanced Wastewater Treatment Plant, 5000 Overlook Ave, SW Washington, DC 20032, USA

^c Department of Civil, Environmental and Architectural engineering, The University of Kansas, KS, USA

^d Research Group of Sustainable Energy, Air and Water Technology, Department of Bioscience Engineering, University of Antwerp, Antwerpen, Belgium.
E-mail: siegfried.vlaeminck@uantwerpen.be

^e Department of Microbiology, University of Innsbruck, Austria

^f ARA Consult GmbH, Innsbruck, Austria

^g Dynamita SARL, France

^h Hampton Roads Sanitation District, VA, USA

ⁱ New Hub, VA, USA

† Electronic supplementary information (ESI) available. See DOI: 10.1039/c9ew00384c

1 Introduction

Deammonification has been the cornerstone for energy-efficient nitrogen removal with the goal being to make wastewater treatment plants energy self-sufficient. Deammonification (partial nitrification/anammox) consists of partial nitrification of ammonium to nitrite through aerobic ammonia-oxidizing bacteria (AerAOB), followed by subsequent removal of the remaining ammonium in combination with the formed nitrite with the help of anoxic ammonium-oxidizing bacteria (AnAOB). The competition for nitrite between AnAOB and NOB is the key challenge in deammonification technologies.¹

Microbial growth is managed by choosing substrate levels which through the Monod relationship determine the overall growth kinetics; hence “kinetic selection” was coined to denote growth rate manipulation.^{2–4} In the case of sidestream deammonification processes, high temperature⁵ and free ammonia (FA) inhibition⁶ in combination with low dissolved oxygen (DO) levels are the predominant mechanisms to manage NOB growth kinetics.⁷ The DEMON® process has been the most widely implemented sidestream deammonification process.^{8,9} DEMON utilizes a pH-driven aeration control at a low dissolved oxygen (DO) set point (0.3 mg O₂ per L) to tightly control the nitrite availability in the reactor while maintaining high residuals of ammonium and alkalinity.^{2,10}

Mainstream conditions do not allow for complete kinetic NOB outselection due to low FA concentrations. Multiple strategies have been proposed, for example bioaugmentation with desirable organisms (*e.g.* sidestream AnAOB and AerAOB) and/or out-selecting of others (*e.g.* NOB).^{2–4} In this way, a maximum growth rate differential between AerAOB and NOB is created to subsequently expose them to “physical selection”, washing NOB out while retaining AerAOB.^{11,12} Tightly controlled levels of ammonium, nitrite and DO are the key to such a growth rate differential. A high ammonium residual (2–5 mg N per L) has been found to be paramount for NOB outcompetition in all process configurations, which can be managed with advanced control strategies like ammonia *versus* NO_x (AvN).^{3,13}

In flocculent mainstream systems, NOB are controlled based on the SRT where the higher maximum growth rate for AerAOB is exploited by reducing the SRT up to the point that NOB wash out.¹⁴ However, AnAOB intrinsically have a low growth rate (0.06–0.21 d^{–1}),^{15,16} which counteracts the SRT control required to wash out NOB in mainstream applications. Suspended growth deammonification systems under sidestream conditions generally require a total SRT of 30–45 days^{7,17} for adequate AnAOB to be present in the system. Because AnAOB prefer to grow in granules, physical selection can exploit this difference in morphology. Physical selection can be achieved based on density using hydrocyclones,¹² size using screens^{11,18} or critical settling velocity in granular technologies like ANAMMOX® and ELAN®.^{19,20} Cyclones and screens are external selectors, typically on the waste activated sludge (WAS) line. The dense or big fraction (‘retained’) is sent back to the reactor from the cyclone or screen, respectively, while the light or small fraction (‘rejected’) is wasted. Cyclones and screens allow for direct management of two morphologies (granules and flocs), and it has been shown for deammonification systems that the retained fraction is the smallest in sludge mass, yet the highest in AnAOB activity, and the rejected fraction is the highest in mass and NOB activity.^{7,14} Physical selectors therefore allow for a more direct management of the microbial conversions and could provide more operational flexibility than feasible in biofilm technologies.

Little is known, however, on how the physical selectors’ activity splits on the process performance and how these interact with kinetic selection under full-scale conditions. While Strass WWTP successfully achieved deammonification in the side- and mainstream lines with the help of physical selectors,^{9,21} this success is not guaranteed, as it results from a complex interplay of several mechanisms. Achieving deammonification, especially in the water line, is feasible only when a balance is found between kinetic selection (NOB out-selection) and physical selection (AnAOB retention). In 2014, the Ejby Mølle wastewater treatment plant in Denmark installed cyclones on the RAS line of the BNR reactor with the aim of increasing settleability and achieving mainstream deammonification. This concept was also combined with bioaugmentation of AnAOB from the sidestream DEMON, similar to the Strass WWTP. However, both goals were challenging due to the long SRT (~30 days) applied, wastewater characterization and reactor conditions. No deammonification was observed despite AnAOB retention with the cyclones and bioaugmentation.^{22,23} Some minor improvements in settleability were achieved at lower SRT, while AnAOB contribution remained questionable.^{22,23} This shows that some core understanding of the process is still lacking, despite ample literature available. Solely applying a mechanism to retain AnAOB does not guarantee AnAOB activity. Mechanistic understanding of the impact of reactor conditions and physical selection parameters is needed to define potential operational windows of success for real-life applications.

In essence, while ample literature is available on ideal conditions to grow and retain AnAOB or out-select NOB, no work has been done on the interactions, trade-offs and potential synergies between kinetic and physical selection. This is important because, as exemplified above, just retaining AnAOB or out-selecting NOB might not be enough to achieve mainstream deammonification. This study relies on a straightforward and easy to apply model which combines steady-state measurements from full-scale physical selectors installed at Strass WWTP with straightforward (steady-state) equations describing both selection types to show how overall and specific selection efficiencies impact both sidestream and mainstream deammonification technologies. Kinetic selection is approached through a minimum Monod function, whereas physical selection was calculated based on a modified sludge washout function. This study mechanistically shows the interactions, trade-offs and potential synergies between kinetic and physical selection for a broad range of conditions. Sensitivity analysis is provided to explore expected trends when selection changes and to provide possible operational windows where further rigorous calibration and validation or expansion of the concept can be tested. The resulting operational window is instrumental to formulating expectations and recommendations for full-scale realization of these deammonification concepts.

2 Materials and methods

2.1 Model development

Growth rates (μ_{AerAOB} , μ_{NOB} and μ_{AnAOB}) were estimated using minimum Monod equations corrected for decay and based on the study of Stewart *et al.* (eqn (1)):⁴

$$\mu_{\text{organism}} = \mu_{\text{max,organism}} \times f_{\text{aer}} \times \min \left(\frac{S_1}{K_{S_1} + S_1}, \dots, \frac{S_n}{K_{S_n} + S_n} \right) - f_{\text{aer}} \times b_{\text{aer}} - (1 - f_{\text{aer}}) \times b_{\text{an}} \quad (1)$$

where $\mu_{\text{max,organism}}$ is the maximum growth rate of AerAOB, NOB or AnAOB (d^{-1}), f_{aer} the aerobic fraction (percentage of reactor's volume that is aerated) (-), S_n the concentration of substrate n (mg per day; $\text{NH}_4\text{-N}$ and DO for AerAOB, $\text{NO}_2\text{-N}$ and DO for NOB, and $\text{NH}_4\text{-N}$ and $\text{NO}_2\text{-N}$ for AnAOB), K_{S_n} the associated half-saturation constant (mg per day) and b the decay rate (d^{-1}). Note that for AnAOB, the factor f_{aer} was replaced by the anoxic fraction ($1 - f_{\text{aer}}$) and an anoxic decay coefficient was used. In addition, decay was only accounted for in the respective zones where growth occurred.

The washout rate of AerAOB, NOB or AnAOB ($1/\text{SRT}_{\text{organism}}$) is given by the sludge mass that is removed by sludge wasting independent of the growth rate, thus inversely proportional to the SRT.²⁴ The external selector induced a split in biomass into a retained and a rejected fraction. The retained fraction is sent back to the WAS line, while the rejected fraction is wasted. The rejection mass split $f_{\text{M,rejected}}$ (%) of the external selection is defined as (eqn (2)):

$$f_{\text{M,rejected}} = \frac{X_{\text{rejected}} \times Q_{\text{rejected}}}{X_{\text{rejected}} \times Q_{\text{rejected}} + X_{\text{retained}} \times Q_{\text{retained}}} = \frac{X_{\text{rejected}} \times Q_{\text{rejected}}}{Q_{\text{selector}} \times X_{\text{selector}}} \quad (2)$$

where X (kg TSS per m^3) is the sludge concentration and Q (m^3 per day) the flow rate of the respective fraction. The waste flow Q_{selector} (m^3 per day) from the reactor with volume V (m^3) to the external selector will therefore have to increase depending on $f_{\text{M,rejected}}$ (%) to reach a similar SRT (d) at a certain recycle ratio (eqn (3)).

$$\begin{aligned} \text{SRT}_{\text{system}} &= \frac{X_{\text{reactor}} \times V}{X_{\text{rejected}} \times Q_{\text{rejected}}} = \frac{X_{\text{reactor}} \times V}{f_{\text{M,rejected}} \times Q_{\text{selector}} \times X_{\text{selector}}} \\ &= \frac{V}{(1 + R) \times f_{\text{M,rejected}} \times Q_{\text{selector}}} \end{aligned} \quad (3)$$

No impact of effluent suspended solids on washout was considered. A schematic of different streams can be found in ESI† A.

To calculate the washout rate for a specific target group of organisms (AerAOB, NOB or AnAOB), an activity balance was calculated over the external selector, which determined the activity retention efficiency η (%). Activity retention efficiency was defined as the percentage of volumetric activity (r_v , kg N

per m^3 per d) measured in the retained fraction of the external selector compared to the total volumetric activity coming in the selector (eqn (4)).

$$\eta_{\text{organism}} = \frac{r_{v,\text{organism,retained}}}{r_{v,\text{organism,retained}} + r_{v,\text{organism,retained}}} \quad (4)$$

The retention efficiency (eqn (4)) can be inserted in the modified SRT equation (eqn (3)) to calculate the organism specific washout rate (eqn (5)):

$$\frac{1}{\text{SRT}_{\text{organism}}} = \frac{(1 - \eta_{\text{organism}})}{\text{SRT}_{\text{system}}} \quad (5)$$

The presence or absence of an organism is ultimately determined by the balance between the growth of the organism and the pressure applied by the washout rate; thus a net growth rate (μ_{net}) can be calculated by subtracting eqn (5) from eqn (1).

2.2 Determination of capacity

Capacity in sidestream systems was defined as the maximum load that can be treated while retaining a 90% $\text{NH}_4^+\text{-N}$ removal efficiency, which can be calculated based on the total inventory of AnAOB (eqn (6)).

$$R_{V,\text{AnAOB}} = \mu_{\text{net,AnAOB}} \left(\frac{\text{SRT}_{\text{AnAOB}}}{\text{HRT}} \right) \left(\frac{(S_o - S_{\text{out}})}{1 + b_{\text{AnAOB}} \times \text{SRT}_{\text{AnAOB}}} \right) \quad (6)$$

Full derivation can be found in ESI† B. As sidestream systems are more granular in nature, capacity was not considered to be limited by sludge loading rates to the clarifiers. In mainstream, this assumption is invalid, thus the increase in capacity was approximated by the percentage difference in total SRT required.

2.3 Fraction of deammonification in mainstream and minimum required AnAOB growth rate

In mainstream deammonification, complete deammonification cannot always be achieved; therefore the degree of deammonification f_{deam} (% total inorganic nitrogen, TIN) was introduced. First, a deammonification rate (in g TIN removed per d) was calculated based on an assumed f_{deam} and the total daily TIN removal calculated by the product of the influent TIN concentration $S_{\text{TIN,in}}$ (g N per m^3), influent flow Q_{in} (m^3 per day), and removal efficiency (%) (eqn (7)):

$$r_{\text{deam}} = f_{\text{deam}} \times Q_{\text{in}} \times S_{\text{TIN,in}} \times \left(1 - \frac{S_{\text{TIN,out}}}{S_{\text{TIN,in}}} \right) \quad (7)$$

The AnAOB rate (g $\text{NH}_4^+\text{-N}$ per d) is calculated based on the deammonification rate, corrected for the TIN to $\text{NH}_4^+\text{-N}$ conversion based on the stoichiometry of AnAOB¹⁶ (eqn (8)). The NOB rate (kg TIN-N per d) was obtained as the TIN

conversion rate that did not go through deammonification (eqn (9)), whereas the AerAOB rate (kg NH₄⁺-N per d) was calculated as the converted TIN load that did not go to AnAOB (eqn (10)).

$$r_{\text{AnAOB}} = r_{\text{deam}} \times \frac{1}{1+1.32} \quad (8)$$

$$r_{\text{NOB}} = Q_{\text{in}} \times S_{\text{TIN}} - r_{\text{deam}} \quad (9)$$

$$r_{\text{AerAOB}} = Q_{\text{in}} \times S_{\text{TIN}} - r_{\text{AnAOB}} \quad (10)$$

Note that only autotrophic metabolisms were considered to limit the number of organisms competing for nitrite. This further allowed the simulation of a “worst-case scenario” where NOB only need to compete with AnAOB for nitrite. Nitrate production and subsequent heterotrophic N removal was not considered and will require COD (present or dosed) to be removed. The AerAOB/NOB ratio was subsequently determined by dividing eqn (10) by eqn (9).

Last, a criterion for sufficient AnAOB growth was determined based on the calculated AnAOB rate. This total rate (in kg NH₄⁺-N per d) can be modified to a volumetric rate (in kg NH₄⁺-N per m³ per d) which can subsequently be inserted into eqn (6).

$$\mu_{\text{min,AnAOB}} = \frac{\left(1 - \frac{S_{\text{TIN,out}}}{S_{\text{TIN,in}}}\right)}{\left(\frac{\text{SRT}_{\text{AnAOB}}}{1 + \text{SRT}_{\text{AnAOB}} \times b_{\text{AnAOB}}}\right)} \quad (11)$$

Full proof of eqn (11) can be found in ESI† C.

2.4 Strass WWTP and physical selectors

The Strass wastewater treatment plant is a two-stage wastewater treatment facility (A/B configuration), treating 250 000 people equivalents.²¹ Produced sludge was anaerobically codigested with food waste, and the filtrate was treated using a DEMON reactor (500 m³).⁹ In 2007, cyclones were installed in the DEMON reactors, operating at 10 m³ per hour and 2 bar inlet pressure. In 2015, the cyclones were replaced by a rotating drum screen with a 52 µm screen size. The “B-stage” mainstream deammonification reactor had cyclones installed in 2011, operating at 20 m³ per hour and 1.8 bar inlet pressure. The cyclone was replaced with a rotating drum screen in 2015 with a 250 µm screen size.

2.5 Activity tests

Specific activity tests were performed on full-scale samples taken from the rejected and retained streams for the screens and cyclones after at least 6 months of operation of these selectors to determine the AnAOB retention efficiencies. Four tests were done in total, two from sidestream sludge (cyclone and screen) and two from the mainstream reactor (cyclone

and screen), to determine the selection efficiencies. Activity tests were performed according to Wett *et al.*²⁵ and Podmirseg *et al.*²⁶ Reactors were operating under steady-state conditions at the time of sampling. Fresh sludge was put in a closed vessel and controlled at 20 °C. Both ammonium and nitrite were spiked to 25 mg N per L. The sludge was aerated for 15 minutes prior to the test to remove any COD present. Next the sludge was purged with N₂ gas to ensure anoxic (DO = 0 mg per day) conditions, whereafter liquid samples were taken every 10 minutes for 1 hour and analyzed for ammonium and NO_x. pH was controlled when necessary. The AnAOB activity was derived from the data using linear regression, fitting the linear part of the activity test. The stoichiometry of ammonium and nitrite removal was checked to be close to theoretical value of 1.32 confirming AnAOB activity rates rather than denitrification.

Ammonium determination is based on derivatization with *o*-phthaldialdehyde/*N*-acetyl-cysteine (OPA/NAC) and fluorescence measurement of the formed isoindols.²⁷ Nitrite and nitrate were quantified by ion pair chromatography with *n*-octylamine as the pairing reagent on a C18 HPLC column and UV-detection at 210 nm according to Doblander and Lackner.²⁸ TSS was measured according to the standard methods.²⁹

As a proxy for the AnAOB abundance and hence activity, heme *c* protein measurements were performed based on the method by Podmirseg *et al.*²⁶ First, 1.5 mL sludge was centrifuged for 3 minutes at 5000 rpm and the supernatant was discarded. The pellet was incubated at 100 °C with 1.5 mL concentrated NaOH for 2 minutes. The mixture was centrifuged again at 5000 rpm for 3 min. After centrifugation, 100 µL Na-dithionite was added and absorbance was measured at 535, 550, 570 nm. The reduced heme compound showed its sharpest peak at 550 nm. Calibration was performed with the 1-heme cytochrome *c* from horse heart. Heme *c* protein levels in biomass were found to be strongly positively correlated with sludge-specific AnAOB rates.²⁶

2.6 Bioaugmentation of sidestream AerAOB and AnAOB into the mainstream system

The full-scale mainstream deammonification reactor was bioaugmented with sidestream sludge. The bioaugmentation rate was calculated as a percentage of the organism's maximum growth rate for this simulation exercise. A bioaugmentation rate of 25% and 17% was assumed for AerAOB and AnAOB, given that 25% of the sidestream reactor's volume is seeded into the mainstream on a weekly basis based on operation data from Strass and a former pilot study.¹⁴ Sidestream AerAOB have been observed to lose some of their activity when introduced into the mainstream reactor. A review on bioaugmentation of autotrophic nitrifiers by Parker and Wanner³⁰ concluded that temperature shock was a major culprit in loss in AerAOB activity. Wett, Jimenez, *et al.*³¹ estimated that 30–50% of the community is active depending on the ammonium residual, while Head and Oleszkiewicz³²

determined that AerAOB lost 58% activity when a temperature shock of 10 °C was induced. Note that bioaugmentation is an exchange of mass; hence the specific activity of the seeded AerAOB will always be greater than that prior to bioaugmentation.^{31,33} For this reason, AerAOB bioaugmentation was assumed to be 50% efficient, reducing the AerAOB bioaugmentation rate to 12.5%. No loss in activity for AnAOB was assumed, as no studies quantifying the activity loss of AnAOB from bioaugmentation from sidestream to mainstream have been published to the authors' knowledge. The bioaugmentation increased the maximum growth rate for AerAOB by 12% (from 0.9 to 1.01 d⁻¹) and for AnAOB by 17% (from 0.100 to 0.117 d⁻¹). All scenarios were bioaugmented unless otherwise stated.

2.7 Model implementation and kinetic parameters

The model output was calculated using Microsoft Excel. The model was thereafter exported to R to allow for 2 or more independent variables to be varied at the same time. Steady state was assumed for all calculations and model outputs.

Maximum growth rates, half-saturation constants, and yields for AerAOB and NOB were taken from the calibrated model in Al-Omari, Wett, *et al.*³⁴ and can be found in ESI† D. The half saturation indices for AnAOB were modified to 0.5 mg N per L for both ammonium and nitrite based on experimental data (data not shown). Kinetic parameters were considered equal for sidestream and mainstream with the excep-

tion of K_o , which was 0.4 and 0.14 mg O₂ per L for AerAOB and NOB, respectively, for mainstream. The K_o values for AerAOB and NOB under sidestream conditions were 0.25 and 0.5 mg O₂ per L, respectively.

3 Results and discussion

3.1 Sidestream deammonification

At Strass WWTP in Austria, the deammonification (DEMON) process was used to treat sidestream water high in ammonium and was operated at a low DO set point based on pH (0.3 mg O₂ per L).⁹ NOB were metabolically out-selected (*i.e.* net growth rate was 0 d⁻¹) because of aeration control used in DEMON, represented by a low anoxic fraction (33%), high free ammonia (1.33 mg N per L), and high temperature (30 °C). This was achieved with the higher K_o for NOB than AerAOB within the model (0.5 vs. 0.25 mg O₂ per L) as confirmed by a previous study by Al-Omari, Wett, *et al.*³⁴ Therefore, only the growth rates for AerAOB and AnAOB are shown in Fig. 1A. The favorable conditions within the sidestream reactor, *i.e.* 100 mg NH₄-N per L residual ammonium, allowed for high growth rates for AnAOB (0.032 d⁻¹), leading to a high retention potential for AnAOB (Fig. 1B).

3.1.1. Impact of cyclones. Cyclones installed on the sidestream achieved a rejection mass split of 80%. Based on steady-state activity balance performed at full scale, an 88% retention efficiency was obtained for AnAOB (Table 1). The cyclones were replaced in 2015 with a rotating drum screen

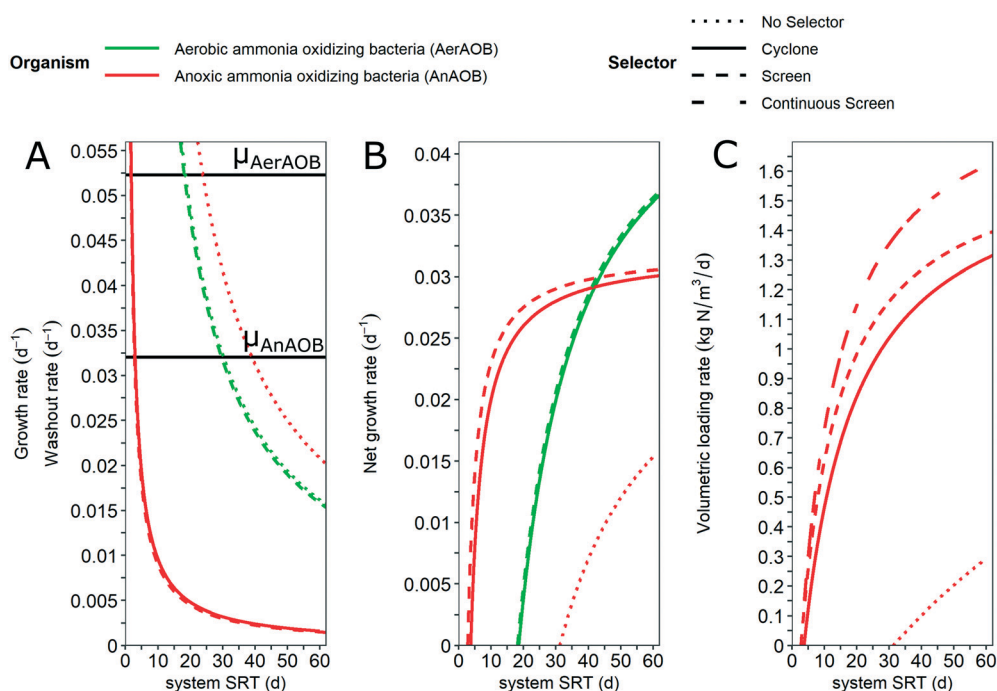


Fig. 1 (A) Growth and washout rate of AerAOB and AnAOB under sidestream conditions (NH₄⁺ = 100 mg N per L, NO₂⁻ = 1 mg N per L, DO = 0.3 mg O₂ per L) with cyclones ($f_{M, rejected} = 0.8$; $\eta_{AnAOB} = 88\%$) and screen ($f_{M, rejected} = 0.7$; $\eta_{AnAOB} = 91\%$). NOB were metabolically outselected (negative growth rate). (B) Selection efficiency achieved at given growth and outselection rates. (C) Volumetric N removal rate by AnAOB in sidestream deammonification with and without external selector based on a 2 day HRT, an incoming ammonium concentration of 1000 mg N per L, and a 90% N-removal rate.

Table 1 AnAOB maximum activity (batch tests, 20 °C), abundance (heme), and mass rejection efficiencies performed on rejected and retained fractions of the screens and cyclones installed on the full-scale sidestream and mainstream deammonification reactors at the wastewater treatment plant in Strass, Austria

Sidestream deammonification		Cyclone		Screen	
Rejected	Specific AnAOB value	0.5	mg NH ₄ ⁺ -N per g VSS per h	5	mAU per g TSS
	Mass split	80%		70%	
	Volumetric AnAOB value	0.4	mg N per L per h	3.5	mAU
Retained	Specific activity	15	mg N per g VSS per h	122	mAU per g TSS
	Mass split	20%		30%	
	Volumetric activity	3	mg N per L per h	82	mAU
AnAOB enrichment		30×		24×	
AnAOB retention efficiency		88%		91%	
Mainstream deammonification		Cyclone		Screen	
Rejected	Specific activity	5.5	mAU per g TSS	4	mAU per g TSS
	Mass split	80%		70%	
	Volumetric activity	4.4	mAU	2.8	mAU
Retained	Specific activity	16	mAU per g TSS	24.5	mAU per g TSS
	Mass split	20%		30%	
	Volumetric activity	3.2	mAU	7.35	mAU
AnAOB enrichment		2.9×		6.1×	
AnAOB retention efficiency		42%		72%	

with 52 µm screen size (270 mesh) and a 70% rejection mass split and obtained a steady-state retention efficiency of 91% for AnAOB. While the enrichment of AnAOB was larger for the cyclone (30×) than for the screen (24×), the screen achieved a higher overall retention efficiency. The screen's smaller rejection mass split meant that more sludge was returned to the reactor, resulting in more AnAOB mass retained. Visually, the retained streams of the screen and cyclones contained larger aggregates than the rejected flows (Fig. F1 and F2 in the ESI†). The selective retention of AnAOB decreased their washout pressure (Fig. 1A), thus increasing their net growth rate (Fig. 1B). At a given total SRT of 30 days, which is the typical operating SRT for a DEMON system,⁹ the effective AnAOB-specific total SRT increased from 30 days without an external selector to 313 and 334 days for the cyclone and screen, respectively. This led to a total capacity of 1.04 kg N per m³ per d (cyclone) and 1.16 kg N per m³ per d (screen) for cyclone and screen, respectively, given a 30 day total system SRT, 2 day HRT, an incoming ammonium concentration of 1000 mg N per L, and a 90% N-removal efficiency (Fig. 1C).

3.1.2. Switch and impact of rotating drum screen. The screen's small edge in AnAOB retention efficiency (3%) increased the treatment capacity of the DEMON reactor by 12%. This allowed for a more intensified operation at a smaller footprint. Alternatively, the SRT could be dropped from 30 days to 22.6 days to match the screen's AnAOB-specific SRT with the cyclone's while still providing the same 90% removal efficiency at similar loads. The excess biomass can be seeded to a mainstream reactor for enhanced mainstream deammonification without sacrificing filtrate treatment efficiency. The washout SRT for AerAOB was calculated to be 18 days (Fig. 1B); thus, preemptive measures should be taken if one wants to retain a healthy AerAOB rate and avoid excess washout. In addition, lamella clarifiers which select

on critical settling velocity, were installed upstream to manage the mass load to the screens and thus minimize the number of flocs sent to the latter. Flocs are compressible and therefore limit the effectiveness of the screen on AnAOB retention. A longer retention time on the screen would be required for the same retention efficiency, limiting the mass load that can be applied.

3.1.3. Implications of enhanced AnAOB retention. Some filtrate streams originating from thermally hydrolyzed (THP) sludge like at the Blue Plains Advanced Wastewater treatment plant in Washington, DC, may have inhibitory compounds in the matrix that limit AnAOB growth.³⁵ For this reason, more AnAOB retention would be increasingly important to safeguard the DEMON's performance when inhibitory compounds are present. Thus, a screen might be advantageous over a cyclone because of the increased AnAOB retention it provides. Zhang, De Clippeleir, *et al.*³⁵ were able to successfully operate a sidestream SBR with THP filtrate at similar loading rates to conventional anaerobic digestion filtrate when AnAOB were selectively retained with a screen and DO was increased to 1 mg O₂ per L to offset colloid-induced mass transfer limitations. However, with no THP at Strass WWTP, the extent of overcoming inhibition was not testable.

Rotating drum screens are, unlike hydrocyclones, not dependent on a specific (constant) flow to achieve the desired separation. The separation is achieved gravitationally and controlled by the liquid level rather than the nozzle pressure. This makes screens more energy efficient (<0.001 kW h m⁻³) than cyclones (0.01–0.1 kW h m⁻³). The ability to operate at differential flows allowed DEMON to operate as a continuous flow system rather than as a sequencing batch reactor (SBR). The continuous DEMON reactor eliminated the need for a settling and decanting phase, saving one hour out of a typical six hour SBR cycle, thus lowering the HRT by 17%. This effectively increased the DEMON system's capacity by an

additional 17% over the SBR with screen installed, netting a total of 29% over a traditional DEMON reactor with cyclones. The ability to operate at a range of flows which the screen provides offers a great perspective for practice as it makes the DEMON process more versatile and robust.

The capacity increase that was achieved with implementation of the continuous DEMON reactor was tested with a stress test and presented in Fig. 2. The loading rate was ramped up from 1 to 1.4 kg N per m³ per d in a 21 day period, whereafter no more filtrate was available to increase the load further. Note that the average filtrate concentration was 1860 ± 50 mg NH₄⁺-N per L, significantly higher than that of typical filtrate (~1000 mg NH₄⁺-N per L), because of codigestion of food waste in the anaerobic digesters. During the ramp-up, both ammonium and TIN removal percentages remained stable at 94 ± 1% and 89 ± 1%, respectively. The theoretically calculated maximum load for the Strass sidestream reactor, given the increased loads due to food waste codigestion, was 2.8 kg N per m³ per d, which was a magnitude greater than the loading rate applied (0.5–1 kg N per m³ per d) in practice for filtrate treatment technologies. During the ramp-up test, the concentration of the filtrate remained the same, and the increase in loading was achieved by gradually increasing the flow from 216 to 311 m³ per day, resulting in an HRT decrease from 1.85 to 1.3 days. This shorter HRT was not incorporated in the capacity calculation eqn (6), which assumed a design HRT of 2 days. Filtrate concentration generally does not change much, given a stable anaerobic digestion performance. An increase in loading will therefore typically be accompanied by a decrease in HRT. As capacity negatively correlated with HRT based on eqn (6), the true

capacity will be lower than the theoretically calculated value based on the initial design. In addition, DEMON reactors operating in SBR mode will have additional loading constraints when HRT, which is managed with volume exchange ratios, is pushed too short. Enough time for settling is required as the sludge bed needs to be settled sufficiently during the decant phase. This potentially puts potential constraints on the MLSS levels in the reactor. Further practical tests will be required to pinpoint what the limiting factor in DEMON installations will be. Despite these hurdles, switching from cyclone to continuous screen operation should achieve an overall 29% net capacity increase.

3.2 Mainstream deammonification

3.2.1. NOB outselection. In mainstream deammonification systems, NOB are not fully kinetically outcompeted and thus need to be considered. Full deammonification may not be realistic given the low substrate concentrations and impact of available carbon for denitrifiers.³⁶ In addition, no AerAOB/NOB activity ratios have been reported above 2–2.5,^{13,36} indicating that complete NOB outselection might not be feasible. A more realistic approach was to assume an *in situ* observed AerAOB/NOB activity rate ratio, which correlates with a percentage of deammonification in the reactor. Han, Vlaeminck, *et al.*¹⁴ showed that mainstream deammonification was achieved at an AerAOB/NOB ratio of 2. This optimal ratio was adapted within the model to reflect a threshold for adequate NOB outselection. Given the operational conditions of the mainstream biological nutrient removal reactor at Blue Plains AWTP (N load = 34 065 kg N per m³ per d,

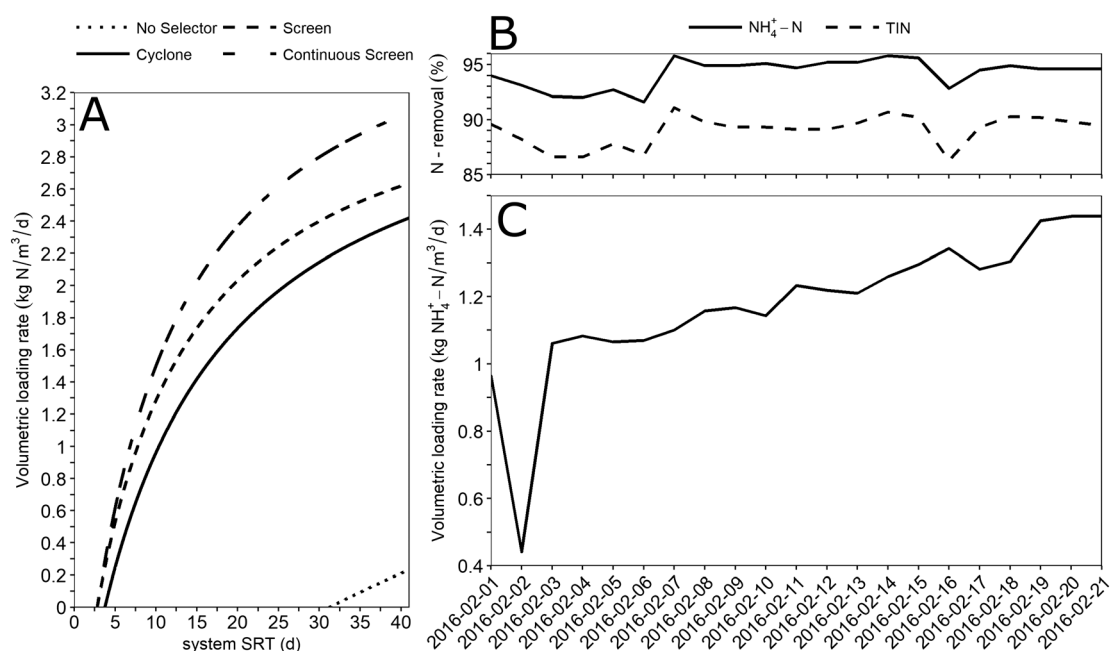


Fig. 2 (A) Stress test performed on continuous sidestream DEMON reactor with screen installed at the wastewater treatment plant in Strass, Austria, to evaluate its maximum capacity. (B) The ammonium and TIN removal percentage during the ramp-up. (C) The loading rate over a three week period achieved by increasing flow rate (average influent NH₄⁺ was 1859 ± 53 mg N per L).

influent TN = 30 mg N per L, and TIN removal = 92%), a 68% deammonification contribution was found to correspond to the previously determined optimal AerAOB/NOB ratio of 2 (Fig. 3D). In addition, heterotrophic denitrifiers were not considered to allow for the worst-case scenario where nitrite not used by AnAOB will be consumed by NOB.

Increasing the ammonium or DO concentrations was beneficial towards kinetically outcompeting NOB independent of the SRT strategy applied because the AerAOB/NOB ratio increased (Fig. 3A and B). High ammonium residuals lowered the dependency of the AerAOB/NOB ratio on low nitrite availability in the aerobic zone. Operation at ammonium residuals greater than 1.5 mg N per L at a DO of 1.5 mg O₂ per L allowed for an AerAOB/NOB ratio greater than 2 at nitrite residuals of 0.5–0.75 mg N per L (Fig. 3A). Similarly, operation at a high DO set point (>1.5 mg O₂ per L) is beneficial when

an ammonium residual of 2 mg N per L was maintained because of the decreased dependency on tight nitrite management (Fig. 3B). High ammonium has been widely cited in the literature to be imperative for mainstream deammonification.^{34,37,38} This study further confirms that high DO is required for flocculent deammonification systems as postulated by Regmi, Miller, *et al.*³⁸

The main goal of kinetic selection was to create a gap in washout SRT between AerAOB and NOB that can be exploited by sludge wasting. Fig. 3E shows the maximum aerobic SRT (AerSRT) that can be applied to ensure an AerAOB/NOB ratio of 2 as a function of the nitrite residual in the aerobic zone for three different ammonium residuals. The higher the maximum AerSRT is, the bigger the eligible AerSRT range. At 0.75 mg NO₂-N per L residual, the maximum SRT was 4, 6, and 10 for 0.5, 1, and 2 mg NH₄-N per L, respectively. This

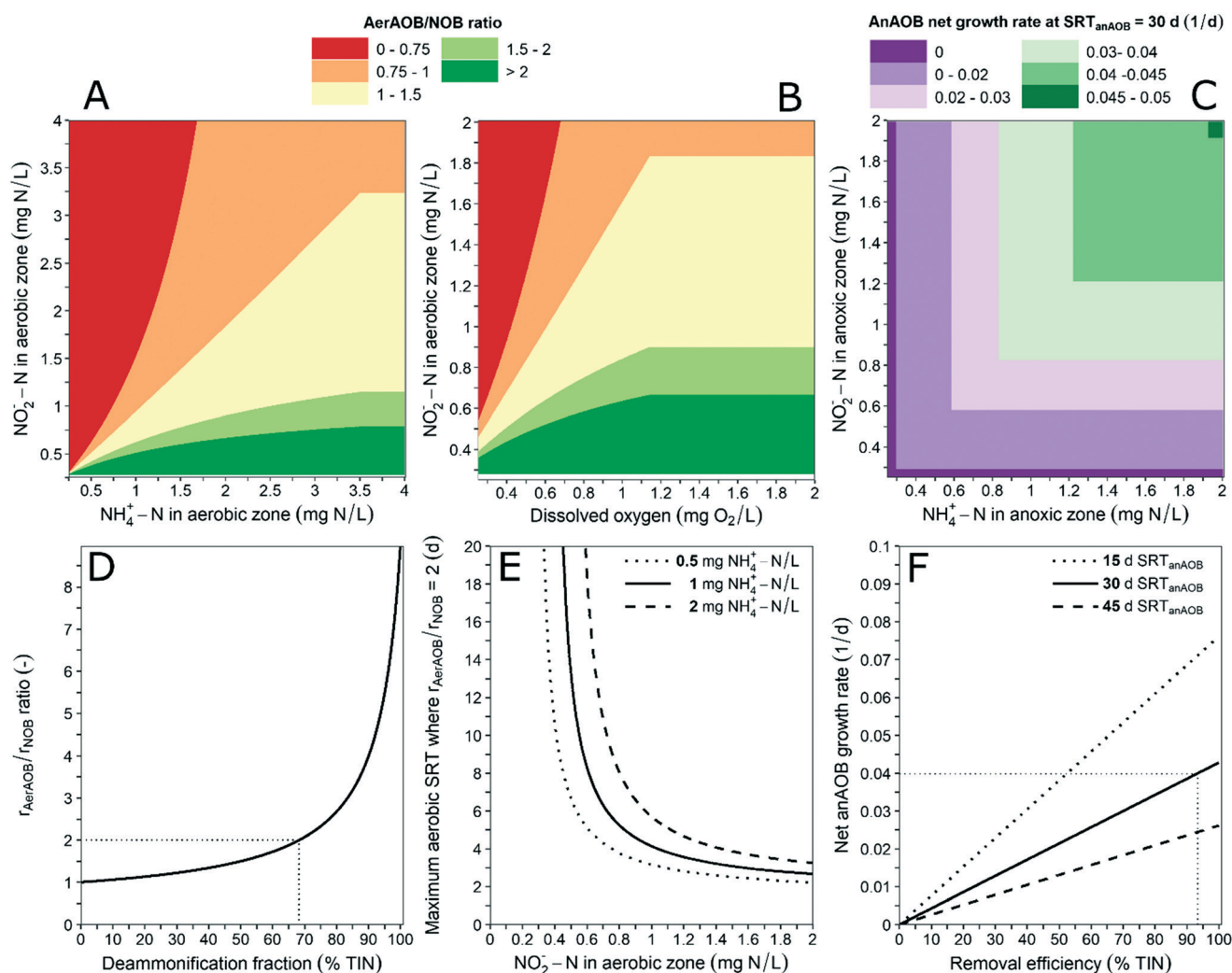


Fig. 3 (A and B) Ratio of intrinsic AerAOB over NOB removal rates as a function of the average concentrations in the reactor's aerobic zones of ammonium and nitrite (A; DO = 1.5 mg O₂ per L) and DO and nitrite (B; ammonium = 2 mg N per L). (C) The net growth rate of AnAOB given an AnAOB-specific SRT of 30 days. (D) Relationship between the percentage of TIN removed through deammonification and the AerAOB/NOB rate ratio in the system. (E) Minimum net AnAOB growth rate required for adequate deammonification given a certain TIN removal for three different AnAOB-specific SRTs. (F) Maximum aerobic SRT where the ratio of AerAOB over NOB removal rates equalled 2 as a function of the average nitrite and ammonium in the aerobic zone.

decreased to 2, 3, and 4 at 2 mg NO₂-N per L for the same respective ammonium residuals. This maximum AerSRT increased with decreasing nitrite concentration in the aerobic zone. However, the impact of the ammonium residual became more significant at lower nitrite concentrations, stressing the importance of managing AerAOB growth.

The best kinetic strategy for deammonification was to shift the focus from creating conditions that hampered NOB growth to creating an environment that favored AerAOB growth. Ammonium and DO are easy to control in a deammonification system with control strategies like ammonium-based aeration control (ABAC)³⁹ or ammonium vs. NO_x (AvN) control.^{3,13} Smart design of the aeration control, like more rapid intermittent aeration (in time or space) as opposed to longer periods, might allow for better management of nitrite.³⁷

3.2.2. AnAOB retention. Next to NOB outselection, AnAOB activity is crucial for the success of mainstream deammonification. The AnAOB in the system should be able to cope with the ammonium loading rate they receive based on the deammonification fraction determined above. This can be approximated by requiring a minimal AnAOB net growth rate in the system to meet a certain TIN removal percentage (Fig. 3F), which is dependent on the AnAOB-specific anoxic SRT (AnSRT). The latter was assumed to be 30 days, which is considered the design operational SRT for many sidestream deammonification systems, thus a relevant target for the AnSRT under mainstream conditions. The minimum net growth rate for AnAOB to maintain a 94% TIN removal was 0.04 d⁻¹ based on the conditions found at Blue Plains AWTP (see section 3.2.1) (Fig. 3F).

The physical selection of AnAOB with screen and cyclone was significantly less efficient in mainstream compared to sidestream deammonification (Table 1). Furthermore, the difference in retention efficiency between screen and cyclone was much more pronounced (72% vs. 42%, respectively). The lower retention efficiencies were most likely the result of a mainstream system being a less ideal environment for AnAOB growth. Mainstream would have a higher percentage of flocs relative to granules, leading to a difference in overall sludge characteristics. A picture of mainstream sludge passed through the screen can be found in Fig. F3 in the ESI.† In ad-

dition, larger nozzle size and screen pore size (250 µm) were required to deal with larger debris found in the mainstream reactor and reduce maintenance. Sidestream, having lower flow rates and less debris, allowed for the installation of a smaller pore size as the risk for clogging was lower. Increasing the retention efficiency or changing the mass split of the external selectors would require changing the selector's specifications, such as decreasing the screen's pore size or installing a smaller nozzle on the cyclone. However, this would also induce challenges in maintenance because more pressure is applied on these selectors. The competitive edge of the screen is dependent on the AnAOB growth within the system, which was limited by nitrite availability. Indeed, as nitrite availability decreased in the reactor, the difference in minimum AnSRT for AnAOB between screen and cyclone increased, indicating that the retention rather than growth was more dominant (Table 2).

The growth of AnAOB was equally dependent on the ammonium and nitrite levels in the anoxic zone, meaning that the lowest substrate determined the growth rate. Given the 30 day AnAOB-specific AnSRT, a minimum ammonium or nitrite level in the anoxic zone of 0.83 mg N per L would be required to meet the 70% deammonification minimum as determined above (Fig. 3C). While higher nitrite residuals would benefit AnAOB growth, they hampered NOB outselection. Maximizing the specific retention of AnAOB (and therefore maximizing its specific SRT) should be prioritized to offset the reduced growth rate. Without any form of AnAOB retention mechanism, the minimum required AnSRT for AnAOB was 48 days for an average nitrite residual of 0.75 mg N per L (Fig. 4A). While this nitrite residual was ideal for NOB outselection (Fig. 4B), the anoxic SRT was too high to be practical. When the nitrite residual was increased, the required SRT became more manageable (35 and 22.5 days for 1 and 2 mg NO₂-N per L, respectively, Fig. 4C and E), but potential for NOB outselection was sacrificed. Physical selectors would therefore be crucial in mainstream application to make simultaneous AnAOB retention and NOB outselection possible. While only two selector types with associated AnAOB activity retentions have been performed within this paper, Fig. 4A, C and E present the full sensitivity of the required SRT over the entire range of AnAOB retention

Table 2 SRT required for a successful mainstream deammonification system given the imposed criteria of an AerAOB/NOB ratio >2, an AnAOB net growth rate of >0.04 d⁻¹, at 20 °C. The AerAOB and NOB retention efficiencies were considered equal at 30%

NO ₂ ⁻ (mg N per L)	AerSRT (d)		Minimum AnSRT (d)		Minimum total SRT (d)			
	Min	Max	Cyclone	Screen	Cyclone		Screen	
			Min	Max	Min	Max		
No bioaugmentation from sidestream								
0.75	2.8	4.8	54.9	26.5	57.7	59.7	60.5	64.5
1	2.4	3.3	33.6	16.2	22.7	24.3	12.2	13.8
2	1.8	2	18.9	9.1	15	15.5	8.1	8.6
With bioaugmentation from sidestream								
0.75	2.8	6.4	27.9	13.5	30.7	34.3	16.3	19.9
1	2.4	4	20.3	9.8	22.7	24.3	12.2	13.8
2	1.8	2.3	13.2	6.3	15	15.5	8.1	8.6

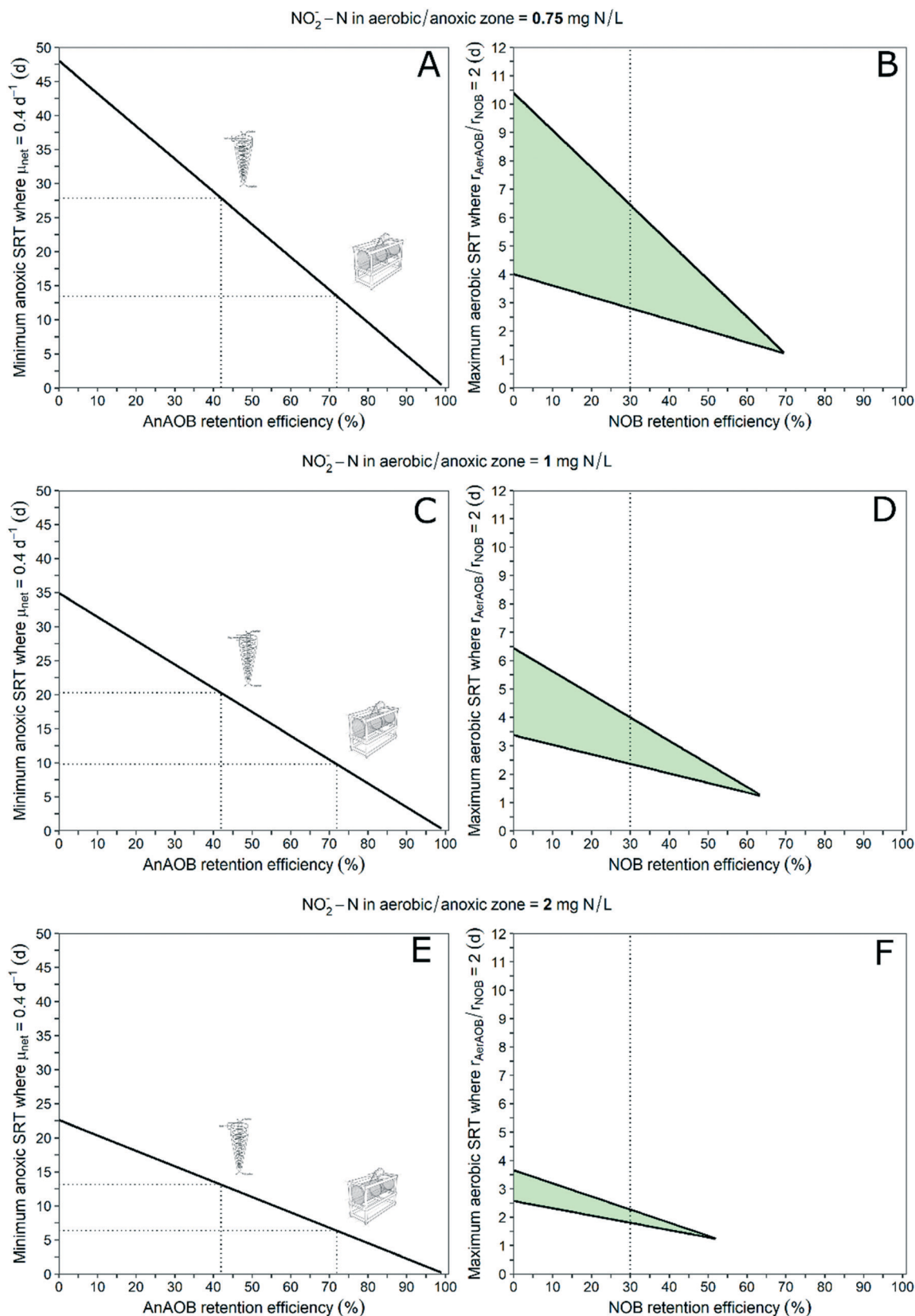


Fig. 4 (A, C and E) Minimum anoxic SRT required to meet the minimum 0.04 d^{-1} AnAOB net growth rate criterion as a function of the AnAOB retention efficiency for an average nitrite residual of 0.75 (A), 1 (C), and 2 (E) mg N per L in the anoxic zone. (B, D and F) The spread of aerobic SRT where operation is possible, given an AerAOB/NOB ratio above or equal to 2 as a function of the NOB retention efficiency for an average nitrite residual of 0.75 (B), 1 (D), and 2 (F) mg N per L in the anoxic zone. The upper boundary of the zone was given by the aerobic SRT where the rate ratio is 2, while the lower boundary is given by the washout SRT of NOB.

efficiencies. This allows plants to narrow down the operational window based on their measurements, thus assessing the feasibility of mainstream deammonification to be calculated for different AnAOB retention efficiencies. Activity measurements would be most suitable as they reflect the actual capability of AnAOB-mediated N removal rather than the mere presence of the organism. Future studies should further detail separation efficiency, backed up with molecular characterization (qPCR) and more heme measurement, as both have been found to correlate very well with AnAOB abundance.²⁶

In addition, more research is needed to optimize the effect of screen size/operation of cyclone on AnAOB retention at certain mixed liquor concentrations. It is known that microbial (sub)communities preferentially grow in small or large flocs depending on the type of organism or operational condition. The migration dynamics of some species, if any, would affect retention and should be investigated in the future. In addition, new installations should be encouraged to acquire retention efficiencies to fine-tune the framework. Finally, plants are encouraged to transfer the concept to their needs and model calibration capabilities,⁴⁰ possibly incorporating more complex model structures to increase the accuracy of predictions.

Bioaugmentation of sidestream sludge (AerAOB + AnAOB) into mainstream further increased the feasibility as it significantly reduces the minimum total SRT (80%, 55%, and 36% for a 0.75, 1, and 2 mg NO₂-N per L residual, respectively); thus, if the plant has a DEMON sidestream facility, bioaugmentation into the mainstream reactor should be a priority to aid mainstream deammonification as this is a typically low-cost capital investment (Table 2). However, bioaugmentation is not a sole recipe for success as it does not *per se* lead to successful deammonification.²³ The full non-bioaugmented scenario can be found in ESI† E. The higher retention efficiency obtained by the screen also directly translated into a higher AnAOB biomass fraction in the reactor. Given the total SRT reported in Table 2, the screen would have 1.8–1.9× the AnAOB biomass in the reactor if both the cyclone and the screen scenario would operate at similar SRT. Alternatively, this meant that the screen allowed operation at total SRTs 1.8–1.9× lower than the cyclone, while having the performance. This shows that, like sidestream, switching from a cyclone to a screen reduces the footprint of the mainstream reactor by 80–90% based on the increase in total SRT, thus intensifying the process by the same amount.

At a nitrite residual of 0.75 mg N per L, the minimum anoxic SRT to achieve 70% deammonification dropped from 28 to 13 days when the cyclone was swapped out with a screen. Once more nitrite was introduced into the system, the required minimum anoxic SRT dropped further as the net AnAOB growth rate increased (Table 2). Increased nitrite residuals also enhanced NOB growth, requiring a more precise and aggressive aerobic SRT control. Maximizing the retention efficiency of AnAOB therefore ensures less dependency on stringent intermittent aeration control for nitrite manage-

ment as it allows for operation at lower nitrite residuals. The screen allowed for the most flexible operation. The efficacy of the external selector is also further influenced by the growth of AnAOB. With increasing nitrite residual, the impact of AnAOB retention decreased as indicated by the decreasing slope in Fig. 4A–E. In addition, the operational SRT range in Table 2 was increasingly narrow the more AnAOB growth was assumed. This means that capacity-limited systems with limited growth will benefit most from the effect of an external selector. Systems with adequate capacity will be able to more loosely control their nitrite residuals.

3.2.3. Excess NOB retention risk. The main function of physical selectors is to retain granular AnAOB. However, some AerAOB and NOB are inadvertently retained due to inefficiencies in the separation step. As long as NOB and AerAOB were retained in a similar way, the NOB outselection strategy was still driven by aeration strategy and aerobic SRT control as discussed in sections 3.2.1 and 3.2.2 (Fig. 4). If more NOB were retained compared to AerAOB, the washout pressure on NOB decreased, counteracting the internal nitrite management. Fig. 4B, D and F show the operational SRT zone where the AerAOB/NOB ratio is equal to or exceeds 2 assuming an AerAOB retention efficiency of 30%. Higher NOB retention efficiencies led to an increased demand for tight SRT control as the operational window decreased. Furthermore, if NOB were retained twice as efficiently as AerAOB, no shortcut nitrogen removal would be possible as the aerobic SRT dropped below 2 days. According to the findings of Han, Vlaeminck, *et al.*,¹⁴ a 30% NOB retention efficiency was deemed the maximum allowable before performance started to deteriorate.

NOB have been reported to stick or migrate to the AnAOB granule's surface when sufficient washout pressure was supplied,¹⁴ linking the AnAOB retention with NOB retention. This could further be managed by operating at slightly higher SRT to avoid migration to the biofilm or apply a harsher shear on the granules in the external selector, which might reduce the AnAOB retention efficiency. AnAOB retention was still key as this also allowed operation at lower nitrite residual, thus aiding the kinetic outselection of NOB rather than a pure SRT-driven one.

4 Conclusions

In conclusion, the balance between kinetic and physical selection is key to both sidestream and mainstream deammonification technologies. This study allowed us to make the following conclusions:

- Screens had superior AnAOB retention over cyclones, this led to a 29% increase in treatment capacity for sidestream and 80–90% increase for mainstream deammonification.
- Superior retention with screens was more emphasized in mainstream compared to sidestream application due to the lower growth rates under these conditions with AnAOB retention efficiencies of 42% and 72% for the cyclone and screen, respectively.

- Maximization of AnAOB retention directly enhanced the success for mainstream deammonification as it decreased its dependency on nitrite residuals.

- Selective NOB retention compared to AerAOB retention decreases the chance for NOB out-selection when using external selectors and increased the importance of tight aerobic SRT control.

- Overall, this paper shows that operation and choice of external selector directly determine the operational strategy and footprint needed to achieve mainstream deammonification. The higher the AnAOB retention and NOB out-selection *via* the physical selector, the lower the need for tight aeration control.

Conflicts of interest

There are no conflicts to declare.

Acknowledgements

This work was supported by the Water Environment and Research Foundation [grant number U1R14] and the National Science Foundation GOALI [grant number 1512667].

References

- 1 S. Agrawal, D. Seuntjens, P. Cocker, S. Lackner and S. E. Vlaeminck, Success of mainstream partial nitrification/anammox demands integration of engineering, microbiome and modeling insights, *Curr. Opin. Biotechnol.*, 2018, **50**, 214–221.
- 2 B. Wett, Method for the treatment of ammonium-containing effluent by means of pH-regulation patent WO/2007/033393, 2007, 29-03-2007.
- 3 B. Wett, A. Al-Omari, P. Regmi, M. Miller, C. Bott and S. Murthy, inventors Method and apparatus for maximizing nitrogen removal from wastewater patent US9346694B2, 2016.
- 4 H. A. Stewart, A. Al-Omari, C. Bott, H. De Clippeleir, C. Su and I. Takacs, *et al.* Dual substrate limitation modeling and implications for mainstream deammonification, *Water Res.*, 2017, **116**, 95–105.
- 5 C. Hellinga, A. Schellen, J. W. Mulder, M. Van Loosdrecht and J. Heijnen, The SHARON process: an innovative method for nitrogen removal from ammonium-rich waste water, *Water Sci. Technol.*, 1998, **37**(9), 135–142.
- 6 A. C. Anthonisen, R. C. Loehr, T. B. S. Prakasam and E. G. Srinath, Inhibition of Nitrification by Ammonia and Nitrous-Acid, *J. - Water Pollut. Control Fed.*, 1976, **48**(5), 835–852.
- 7 B. Wett, M. Hell, G. Nyhuis, T. Puempel, I. Takacs and S. Murthy, Syntrophy of aerobic and anaerobic ammonia oxidisers, *Water Sci. Technol.*, 2010, **61**(8), 1915–1922.
- 8 S. Lackner, E. M. Gilbert, S. E. Vlaeminck, A. Joss, H. Horn and M. C. van Loosdrecht, Full-scale partial nitrification/anammox experiences—an application survey, *Water Res.*, 2014, **55**, 292–303.
- 9 B. Wett, Development and implementation of a robust deammonification process, *Water Sci. Technol.*, 2007, **56**(7), 81–88.
- 10 B. Wett, Solved upscaling problems for implementing deammonification of rejection water, *Water Sci. Technol.*, 2006, **53**(12), 121.
- 11 S. Murthy, E. Giraldo, N. Dockett and W. Bailey, Method and apparatus for wastewater treatment using screens patent US2014/0131273A1, 2014.
- 12 B. Wett, G. Nyhuis, C. Cyprien, M. Hell, I. Takács and S. Murthy, *Development of enhanced deammonification selector*, WEFTEC10, New Orleans, 2010.
- 13 P. Regmi, R. Bunce, M. W. Miller, H. Park, K. Chandran and B. Wett, *et al.* Ammonia-based intermittent aeration control optimized for efficient nitrogen removal, *Biotechnol. Bioeng.*, 2015, **112**(10), 2060–2067.
- 14 M. Han, S. E. Vlaeminck, A. Al-Omari, B. Wett, C. Bott and S. Murthy, *et al.* Uncoupling the solids retention times of flocs and granules in mainstream deammonification: A screen as effective out-selection tool for nitrite oxidizing bacteria, *Bio-resour. Technol.*, 2016, **221**, 195–204.
- 15 T. Lotti, R. Kleerebezem, C. Lubello and M. C. van Loosdrecht, Physiological and kinetic characterization of a suspended cell anammox culture, *Water Res.*, 2014, **60**, 1–14.
- 16 M. Strous, J. J. Heijnen, J. G. Kuenen and M. S. M. Jetten, The sequencing batch reactor as a powerful tool for the study of slowly growing anaerobic ammonium-oxidizing microorganisms, *Appl. Microbiol. Biotechnol.*, 1998, **50**(5), 589–596.
- 17 A. Joss, N. Derlon, C. Cyprien, S. Burger, I. Szivak and J. Traber, *et al.* Combined nitrification-anammox: advances in understanding process stability, *Environ. Sci. Technol.*, 2011, **45**(22), 9735–9742.
- 18 B. Wett, C. Bott, S. Murthy and H. De Clippeleir, Method and apparatus for wastewater treatment using external selection patent US9,670,083B2, 2017.
- 19 T. Lotti, R. Kleerebezem, Z. Hu, B. Kartal, M. K. de Kreuk and C. van Erp Taalman Kip, *et al.* Pilot-scale evaluation of anammox-based mainstream nitrogen removal from municipal wastewater, *Environ. Technol.*, 2015, **36**(9–12), 1167–1177.
- 20 NOB activity suppression in the anammox based process ELAN applied to the water line of a WWTP, *11th IWA Leading Edge Technology Conference*, ed. A. Val del Río, N. Morales, J. Vázquez-Padín, R. Fernández-González, J. Campos and A. Mosquera-Corral, *et al.*, 2014.
- 21 B. Wett, S. M. Podmirseg, M. Gomez-Brandon, M. Hell, G. Nyhuis and C. Bott, *et al.* Expanding DEMON Sidestream Deammonification Technology Towards Mainstream Application, *Water Environ. Res.*, 2015, **87**(12), 2084–2089.
- 22 J. Sandino, A. Willoughby, D. Houweling, L. Havsteen, P. Nielsen and T. Constantine, Improved settleability in a BNR process from hydrocyclone-induced biomass granulation, *Proceedings of the Water Environment Federation*, 2016, **2016**(11), 4688–4696.
- 23 N. Uri, P. H. Nielsen, A. Willoughby, L. Downing, Z. Li and K. Chandran, Modelling the Selective Retention of PAOs and

- Nitrospira (Comammox?) in a Full-Scale Implementation of WAS Hydrocyclones at the Ejby Mølle WWTP, *Proceedings of the Water Environment Federation*, 2017, 2017(7), 4079–4084.
- 24 E. Metcalf, *Wastewater Engineering: Treatment and Reuse*, McGraw-Hill Education, 2003.
 - 25 B. Wett, S. Murthy, I. Takács, M. Hell, G. Bowden and A. Deur, *et al.* Key Parameters for Control of DEMON Deammonification Process, *Water Practice*, 2007, 1(5), 1–11.
 - 26 S. M. Podmirseg, T. Pempel, R. Markt, S. Murthy, C. Bott and B. Wett, Comparative evaluation of multiple methods to quantify and characterise granular anammox biomass, *Water Res.*, 2015, 68, 194–205.
 - 27 S. M. Lloret, J. V. Andres, C. M. Legua and P. C. Falco, Determination of ammonia and primary amine compounds and Kjeldahl nitrogen in water samples with a modified Roth's fluorimetric method, *Talanta*, 2005, 65(4), 869–875.
 - 28 C. Doblander and R. Lackner, Metabolism and detoxification of nitrite by trout hepatocytes, *Biochim. Biophys. Acta*, 1996, 1289(2), 270–274.
 - 29 APHA, *Standard methods for the examination of the water and wastewater*, Washington, DC, 2005.
 - 30 D. Parker and J. Wanner, Review of Methods for Improving Nitrification through Bioaugmentation, *Water Practice*, 2007, 1(5), 1–16.
 - 31 B. Wett, J. A. Jimenez, I. Takács, S. Murthy, J. R. Bratby and N. C. Holm, *et al.* Models for nitrification process design: one or two AOB populations?, *Water Sci. Technol.*, 2011, 64(3), 568–578.
 - 32 M. A. Head and J. A. Oleszkiewicz, Bioaugmentation for nitrification at cold temperatures, *Water Res.*, 2004, 38(3), 523–530.
 - 33 S. M. Podmirseg, M. A. Schoen, S. Murthy, H. Insam and B. Wett, Quantitative and qualitative effects of bioaugmentation on ammonia oxidisers at a two-step WWTP, *Water Sci. Technol.*, 2010, 61(4), 1003–1009.
 - 34 A. Al-Omari, B. Wett, I. Nopens, H. De Clippeleir, M. Han and P. Regmi, *et al.* Model-based evaluation of mechanisms and benefits of mainstream shortcut nitrogen removal processes, *Water Sci. Technol.*, 2015, 71(6), 840–847.
 - 35 Q. Zhang, H. De Clippeleir, C. Su, A. Al-Omari, B. Wett and S. E. Vlaeminck, *et al.* Deammonification for digester supernatant pretreated with thermal hydrolysis: overcoming inhibition through process optimization, *Appl. Microbiol. Biotechnol.*, 2016, 100(12), 5595–5606.
 - 36 M. Han, H. De Clippeleir, A. Al-Omari, B. Wett, S. E. Vlaeminck and C. Bott, *et al.* Impact of carbon to nitrogen ratio and aeration regime on mainstream deammonification, *Water Sci. Technol.*, 2016, 74(2), 375–384.
 - 37 P. Regmi, M. W. Miller, B. Holgate, R. Bunce, H. Park and K. Chandran, *et al.* Control of aeration, aerobic SRT and COD input for mainstream nitrification/denitrification, *Water Res.*, 2014, 57, 162–171.
 - 38 P. Regmi, M. Miller, S. Murthy and C. Bott, inventors Method and apparatus for maximizing nitrogen removal from wastewater patent US2014/0263041A1, 2014.
 - 39 L. Rieger, R. M. Jones, P. L. Dold and C. Bott, Ammonia-Based Feedforward and Feedback Aeration Control in Activated Sludge Processes, *Water Environ. Res.*, 2014, 86(1), 63–73.
 - 40 G. Sin, S. W. Van Hulle, D. J. De Pauw, A. van Griensven and P. A. Vanrolleghem, A critical comparison of systematic calibration protocols for activated sludge models: a SWOT analysis, *Water Res.*, 2005, 39(12), 2459–2474.