

A Turbulence-Resolving Numerical Investigation of Wave-Supported Gravity Flows

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Key Points:

- For typical bed slope and wave condition in the active margin, wave-supported gravity flows over flat bed are transitionally turbulent
- Simulated gravity flow has a speed of about $1 \sim 2$ cm/s and sediment mass concentration is no more than 30 kg/m^3
- Wave direction can change the resulting gravity current speed by about a factor of two

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15 Abstract

16 Wave-supported gravity flows (WSGFs) have been identified as a key process driving the
17 offshore delivery of fine sediment across continental shelves. However, our understand-
18 ing on the various factors controlling the maximum sediment load and the resulting grav-
19 ity current speed remains incomplete. We adopt a new turbulence-resolving numerical
20 model for fine sediment transport to investigate the formation, evolution and termina-
21 tion of WSGFs. We consider the simplest scenario in which fine sediments are supported
22 by the wave-induced fluid turbulence at a low critical shear stress of erosion over a flat
23 sloping bed. Under the energetic wave condition reported on Northern California Coast
24 with a shelf slope of 0.005, simulation results show that WSGFs are transitionally tur-
25 bulent and that the sediment concentration cannot exceed 30 kg/m^3 (g/L) due to the
26 attenuation of turbulence by the sediment-induced stable density stratification. Wave
27 direction is found to be important in the resulting gravity current intensity. When waves
28 are in cross-shelf direction, the downslope current has a maximum velocity of 1.2 cm/s,
29 which increases to 2.1 cm/s when waves propagate in the along-shelf direction. Further
30 analysis on the wave-averaged momentum balance confirms that when waves are par-
31 allel to the slope (cross-shelf) direction, the more intense wave-current interaction re-
32 sults in larger wave-averaged Reynolds shear stress and thus in a smaller current speed.
33 Findings from this study suggest that the more intense cross-shelf gravity current ob-
34 served in field may be caused by additional processes, which may enhance the sediment-
35 carrying capacity of flow, such as the ambient current or bedforms.

36 **Plain Language Summary**

37 Fine sediments delivered by rivers are the main agent to carry terrestrial organic car-
38 bon, nutrients and contaminants to the deep ocean. Therefore, it is important to under-
39 stand and further predict the fate of these fine sediments in the coastal ocean. Recent
40 field observations have revealed that through resuspension by waves, the thin wave bot-
41 tom boundary is a main offshore delivery pathway of fine sediment, through a process
42 called wave-supported sediment-driven gravity flows. This study uses a turbulence-resolving
43 numerical model for fine sediment resuspension in the wave bottom boundary layer to
44 simulate the wave-supported gravity flows driven by energetic wave conditions that are
45 observed in active margins. Model results allow us to provide a constraint on the max-
46 imum offshore sediment flux for flat bed condition and the uncertainty due to wave di-
47 rection is also addressed. Compared with field observations, findings from this model study
48 indicate that other key factors, such as bedforms and superimposed currents, may play
49 a role to enhance the offshore sediment flux.

50 1 Introduction

51 Identification of the physical processes driving substantial offshore transport of fine
 52 terrestrial sediments across the continental shelves is the key to the overall understand-
 53 ing and prediction of sediment source to sink (Wright & Nittrouer, 1995). Fine sediment
 54 transport in coastal ocean is driven by a variety of processes, such as tidal currents, wind-
 55 generated currents, circulation by density gradients, hypopycnal or hyperpycnal river plumes,
 56 turbidity currents and wave bottom boundary layer (WBBL) flows (Nittrouer & Wright,
 57 1994). Typical shelf currents are mostly directed parallel to the coast and hence are the
 58 main drivers of along-shelf sediment transport (Nittrouer & Wright, 1994), while the shelf
 59 slope is usually too mild to drive auto-suspending turbidity currents. Field observations
 60 at Northern California Shelf near Eel River revealed that even during river flooding events,
 61 most sediments delivered directly from the river plume were deposited on the inner shelf
 62 (Geyer et al., 2000). Therefore, the main mechanisms responsible for the offshore deliv-
 63 ery of fine sediment have been associated with the resuspension by wave orbital motions
 64 (Harris & Wiberg, 2002; Wright & Friedrichs, 2006).

65 Our general understanding of wave-driven resuspension of sediments from the sea
 66 floor is due to wave stirring and the resulting offshore transport is parameterized by wave
 67 energy gradient across the continental shelf (Harris & Wiberg, 2002). Field observations
 68 in STRATAFORM program further revealed the existence of wave-supported gravity flows
 69 (WSGFs) as a viable mechanism driving persistent offshore transport of fine sediment
 70 in the coastal ocean (Ogston et al., 2000; Traykovski et al., 2000). These flows have been
 71 observed to be a cross-shelf near-bed density current due to highly concentrated fine sed-
 72 iment suspension over a sloping bed. Because of the gentle slope of continental shelves,
 73 the resulting buoyancy anomaly confined in the thin WBBL near seabed can only drive
 74 a low speed cross-shelf current (several cm/s, see Traykovski et al. (2000, 2007)). How-
 75 ever, using a simple parameterization of WSGF, Scully et al. (2003) estimated that about
 76 26% of fine sediment delivered by Eel River to the mid-shelf during flood season was through
 77 WSGF. By incorporating a WBBL module to model WSGF in the regional-scale ocean
 78 model ECOM-SED, Harris et al. (2005) further showed that when WSGF was neglected,
 79 their model cannot predict the observed mid-shelf depositional pattern at Eel River Shelf.
 80 Thus, clear understanding and better parameterization of WSGF are necessary for study-
 81 ing the offshore sediment transport.

82 Although the importance of WSGF has been recognized, the magnitude of the cor-
83 responding cross-shelf sediment transport and the physical parameters that control it
84 remain unclear. A key uncertainty of the transport lies in the magnitude of the cross-
85 shelf (downslope) gravity current speed. A literature survey suggests that, although WS-
86 GFs have been observed in many continental shelves (e.g., Hale and Ogston (2015); Jaramillo
87 et al. (2009); Traykovski et al. (2000, 2007, 2015)), the cross-shelf current speed differs
88 by several factors, ranging from a few to tens of cm/s. Moreover, different from typical
89 turbidity current, field data show that WSGF requires persistent wave energy to gen-
90 erate sufficient fluid turbulence in the WBBL, which supports the suspended sediments
91 (Hale & Ogston, 2015; Traykovski et al., 2000). There are many physical variables that
92 can affect the dynamics of WSGF. In addition to the wave forcing and shelf slope, whose
93 maximum values are widely reported by field observations (Traykovski et al., 2000), other
94 variables, such as the ambient current (Ma et al., 2008) and bedforms (Traykovski et al.,
95 2015), may have the capability to enhance the sediment-carrying capacity (maximum
96 sediment load) of flow and the downslope gravity flow speed. Laboratory experiments
97 also confirmed that, when only about 13% of very fine sand was present in mud, small
98 bedforms appeared which enhance the wave boundary layer turbulence (Hooshmand et
99 al., 2015).

100 There are several challenges to directly resolve WSGF in the regional-scale mod-
101 eling of sediment source to sink. Firstly, typical ocean models are formulated in terms
102 of wave-averaged variables and hence the intra-wave processes are not resolved. As a re-
103 sult, a WBBL module or parameterizations for WSGF using the averaged momentum
104 balance and equilibrium mass balance are needed (Harris et al., 2005; Scully et al., 2003;
105 Wright et al., 2001). Secondly, field observations all indicate that WSGFs occur primar-
106 ily in the WBBL, whose thickness is only about 10 cm. A way to obtain a better under-
107 standing of WSGF is to employ a turbulence resolving model to gain insight into tur-
108 bulence and fine sediment transport in the transitionally turbulent WBBL. At the most
109 energetic condition where WSGFs have been observed, previous turbulence-resolving nu-
110 mercial simulations of sediment-laden WBBL (without a slope) indicated that the WBBL
111 is transitionally turbulent, meaning that the flow is only turbulent during a portion of
112 the wave period (Cheng et al., 2015; Ozdemir et al., 2010). Furthermore, it was found
113 that the sediment-induced stable density stratification attenuated flow turbulence when
114 a sufficient amount of fine sediment was available. This caused the formation of a luto-

cline, a region of sharp negative sediment concentration gradient, which effectively confined fine sediments within the thin wave boundary layer, consistent with field observed WSGF events (Traykovski et al., 2000, 2015). More importantly, their simulation results showed that when enough sediments were introduced to WBBL (either through initially prescribed sediment load or resuspension from the bottom), the flow became laminar. In other words, at a given sediment settling velocity and wave intensity, there exists a maximum sediment load (or sediment-carrying capacity). Beyond this limit, turbulence in the wave boundary layer is significantly attenuated and the flow becomes laminar. As a result, the WBBL can no longer sustain more sediments.

Insights into sediment-laden WBBL by turbulence-resolving simulations can be extended to simulate WSGF by including a gentle bottom slope. Ozdemir (2016) showed that for a shelf slope of 0.005, the peak downslope velocity was achieved at the sediment carrying capacity of flow but the value was only about 0.8 cm/s. Although this downslope velocity is lower than that observed in the field, Ozdemir (2016)'s work is important as its finding implies that other physical factors not investigated by his numerical model could enhance the downslope gravity current. In the model of Ozdemir (2016), the main control for the speed of downslope gravity flow is the prescribed constant suspended sediment load. In reality, the sediment load in WBBL is a variable dictated by bottom resuspension and deposition. In this study, we present a newly developed turbulence-resolving numerical model, which is able to efficiently simulate WSGFs with the sediment resuspension and deposition capability. To continue the work of Ozdemir (2016), we investigate the transitional turbulent flow features of WSGF and address the physical factor of wave direction in WSGF.

The goal of this study is to understand the intensity of WSGFs under the energetic wave conditions similar to Northern California continental shelf using a turbulence-resolving numerical model with a more realistic resuspension/deposition capability. The specific objectives are to 1) understand the transitional turbulent flow characteristics of the resulting WSGF over an erodible flat bed, 2) investigate the generation, evolution and termination of WSGFs, particularly regarding the mechanics controlling the downslope gravity current, 3) compare the simulation results with existing field observations and parameterizations in regional-scale modeling, in order to improve the understanding and modeling of WSGFs. The remaining of this paper is organized as follows. Model formulation, design of numerical experiments and method used for data analysis are presented

148 in section 2. Section 3 contains main model results of WSGFs. Intra-wave flow charac-
 149 teristics and transitionally turbulent flow features are shown in subsection 3.1. The re-
 150 sulting cross-shelf currents and sediment-laden WBBLs over a sloping bed are presented
 151 in subsection 3.2. The mechanism governing the downslope gravity driven flow in WSGF
 152 are studied in subsection 3.3. Discussions on the comparison with field observations and
 153 on the parameterization of WSGFs are given in section 4. Conclusions are given in sec-
 154 tion 5.

155 **2 Methodology**

156 **2.1 Model Domain**

157 As one kind of seafloor gravity currents, the WSGF is simplified in this study to
 158 be a WBBL problem using the boundary layer approximation, which is appropriate for
 159 relatively long surface wave with small amplitude (typical on continental shelf, J. Trow-
 160 bridge and Madsen (1984)). An idealized computational domain covering the WBBL over
 161 an erodible flat bed (Figure 1) is used. In a coordinate system with its origin in the bot-
 162 tom corner, the rectangular domain has a size of $L_1 \times L_2 \times L_3$ in the downslope (or cross-
 163 shelf x_1), the cross-slope (or along-shelf x_2) and the bed-normal (x_3) direction, respec-
 164 tively. For numerical experiments of WSGF, the small bottom slope $\tan(\theta)$ is always spec-
 165 ified in x_1 -direction (illustrated in Figure 1).

166 The formation of WSGF is due to a significant amount of fine sediment suspended
 167 by the near-bed wave orbital motion while the ambient current is relatively weak (Traykovski
 168 et al., 2000). In this work, we seek for a numerical solution of WSGF in statistically steady
 169 state under the energetic wave condition in Northern California continental shelf near
 170 Eel River where WSGFs were observed. In our simulations, the boundary layer flow is
 171 driven by a time-oscillatory pressure gradient, which is uniform in the bed-normal di-
 172 rection. The resulting free-stream above the turbulent WBBL mimics the wave orbital
 173 motion caused by long waves, which is idealized to have a simple sinusoidal velocity. The
 174 intensity of oscillatory flow is uniquely characterized by the amplitude of free-stream ve-
 175 locity, while the wave direction is specified either in the cross- or along-shelf direction
 176 in order to investigate its effect on the resulting WSGFs.

177 In the resulting WBBL, the near-bed wave orbital motion exerts a shear stress on
 178 the erodible flat bed. When the bottom shear stress exceeds the critical shear stress of

erosion, sediments are eroded and enter the computational domain. We assume that the sediments in suspension are monodispersed, which are supported by the wave-induced fluid turbulence during WSGF events. Following the boundary layer approximation, the two-phase (water and sediment) flow is regarded to be statistically homogeneous in the cross-shelf (x_1) and along-shelf (x_2) directions, where periodic boundary conditions are implemented. For a given wave condition, the suspended sediment load is controlled by the resuspension and deposition of bed sediments (Cheng et al., 2015). Due to the relatively small settling velocity of fine sediment considered here, turbulence in the WBBL constantly suspends a considerable amount of sediment in the model domain.

According to earlier numerical studies of fine sediment transport in WBBL without a bottom slope (Cheng et al., 2015; Ozdemir et al., 2010), when enough sediments are suspended in the domain, the sediment suspension is confined within a thin layer close to the bed due to the sediment-induced stable density stratification. When the WBBL has a gentle bottom slope (specified in x_1 direction), the near-bed density anomaly caused by the sediment suspension creates a persistent gravitational force in the downslope direction, which drives a wave-averaged cross-shelf current (illustrated in Figure 1). The resulting WSGF reaches an equilibrium state when the downslope gravitational force balances the vertical distribution of the total shear stress in the water column (Parsons et al., 2009). Moreover, there exists a maximum sediment load for a given WBBL due to the significant attenuation of turbulence induced by sediment. Consequently, the cross-shelf gravity current has a maximum speed when the suspended sediment load reaches the carrying capacity of flow. We design numerical experiments in section 2.4 to investigate this maximum current speed.

2.2 Governing Equations and Boundary Conditions

2.2.1 The Two-phase Flow Eulerian Method

The equilibrium Eulerian approach (Balachandar & Eaton, 2010) has been widely used in the study of dilute fine sediment transport (Cheng et al., 2015; Ozdemir et al., 2010; Shringarpure et al., 2012). Under the assumption that the fine sediment particles in water have negligible inertia, the suspended sediments can follow the local carrier flow closely (Ferry et al., 2003). Consequently, the velocity field of sediment phase v_i is de-

209 determined from the velocity field of carrier flow u_i and the particle settling velocity w as

210
$$v_i = u_i + w n_i. \quad (1)$$

211 For clarity, tensor notation is utilized hereafter and the subscript $i = 1, 2, 3$ corresponds
 212 to the downslope (cross-shelf), cross-slope (along-shelf) and bed-normal direction, re-
 213 spectively. In equation (1), n_i is a normalized gravity vector representing the direction
 214 of the gravitational acceleration. In the adopted coordinate system (see Figure 1), it reads
 215 $n_i = \begin{bmatrix} \sin \theta & 0 & -\cos \theta \end{bmatrix}$. The application of the equilibrium Eulerian approximation
 216 simplifies the full Eulerian two-phase flow formulation by avoiding solving the particle
 217 momentum equations.

218 **2.2.2 Fluid Phase**

219 For dilute sediment transport in water, the Boussinesq approximation is valid to
 220 simplify the governing equations for carrier flow phase. Subject to the continuity equa-
 221 tion $\partial u_i / \partial x_i = 0$, the incompressible Navier-Stokes momentum equations for carrier
 222 flow read (Shringarpure et al., 2012)

223
$$\frac{\partial u_i}{\partial t} + u_j \frac{\partial u_i}{\partial x_j} = -\frac{1}{\rho} \frac{\partial p}{\partial x_i} + \nu \frac{\partial^2 u_i}{\partial x_j \partial x_j} + Rg\phi n_i + S_i, \quad (2)$$

224 where ρ is the water density, ν is the fluid kinematic viscosity, g is the gravitational ac-
 225 celeration constant, $R = 1.65$ is the submerged specific gravity of sediment, p is the pres-
 226 sure field, and ϕ denotes the volumetric concentration of sediment. In equation (2), the
 227 buoyancy (third) term on right-hand side (RHS) accounts the coupling-effects from sed-
 228 iment phase. The last term S_i represents the prescribed time-variant pressure gradients
 229 for the generation of oscillatory flows, which is written as

230
$$S_i = U_w \Omega_w \cos(\varphi) m_i, \quad (3)$$

231 where U_w is the amplitude of free-stream velocity, $\Omega_w = 2\pi/T_w$ is the wave angular fre-
 232 quency with T_w represents the wave period, $\varphi = \Omega_w t$ is the wave phase and m_i (see
 233 Table 1) is the wave direction vector. By using equation (3), the free-stream orbital mo-
 234 tion is expressed as a monochromatic and perfectly symmetric time series of $u_w(t) =$
 235 $U_w \sin(\varphi)$. In this study, we simulate the same wave condition with different wave di-
 236 rections specified by m_i , while the bottom slope is fixed in the x_1 (cross-shelf) direction.

237 The computational domain has a shear-free top boundary where the free-slip, rigid-
 238 lid boundary condition is implemented, which reads

$$239 \quad \frac{\partial u_1}{\partial x_3} = 0, \quad \frac{\partial u_2}{\partial x_3} = 0, \quad u_3 = 0 \quad \text{at } x_3 = L_3. \quad (4)$$

240 Due to this free-slip treatment at top of the domain, the entrainment of ambient fluid
 241 leads to a slow development in the wave-averaged current above the lutocline where flow
 242 is nearly laminar (Shringarpure et al., 2012). However, this slow increase in mean cur-
 243 rent above the lutocline almost has no impact on the near-bed gravity flow and its ef-
 244 fect is assumed to be negligible. In contrast, the bottom of the computational domain
 245 is modeled as an erodible bed and the no-slip boundary condition is applied for the fluid
 246 velocities, which is written as

$$247 \quad u_i = 0 \quad \text{at } x_3 = 0. \quad (5)$$

248 2.2.3 Sediment Phase

249 Derived from the principle of mass conservation, the resulting advection-diffusion
 250 equation for the volumetric concentration of sediment is written as (Shringarpure et al.,
 251 2012)

$$252 \quad \frac{\partial \phi}{\partial t} + \frac{\partial (\phi v_j)}{\partial x_j} = K \frac{\partial^2 \phi}{\partial x_j \partial x_j}, \quad (6)$$

253 where K is the effective diffusivity of sediment. Similar as in previous studies (Cheng
 254 et al., 2015; Ozdemir et al., 2010), the Schmidt number $Sc = \nu/K$ is specified to be 0.5.
 255 Note that the sediment phase velocity v_j is calculated using equation (1) from the car-
 256 rier flow velocity and the particle settling velocity.

257 For the sediment phase, the no-flux boundary condition is applied at the top of the
 258 domain. This condition imposes no net transport of sediment across the top boundary
 259 throughout the computation, which reads (Ozdemir et al., 2010)

$$260 \quad \phi w n_3 - K \frac{\partial \phi}{\partial x_3} = 0 \quad \text{at } x_3 = L_3. \quad (7)$$

261 At the bottom of computational domain, the erodible/depositional boundary condition
 262 (Cheng et al., 2015; Nelson & Fringer, 2018) is implemented, which is written as

$$263 \quad \phi w n_3 - K \frac{\partial \phi}{\partial x_3} = q_e + q_d \quad \text{at } x_3 = 0, \quad (8)$$

264 where q_e and q_d are the erosional and depositional fluxes at the bottom, respectively. Fol-
 265 lowing the continuous deposition formulation (Sanford, 2008), the depositional flux is

modeled as $q_d = \phi w n_3$. Since the sediment concentration ϕ is calculated in every numerical time step, the depositional flux is a model variable depending on the last flow condition in the domain. The erosional flux is calculated using the Partheniades-Ariathurai-type formulation (Sanford & Maa, 2001), which has the following expression

$$q_e = \begin{cases} m_e \left(\frac{|\tau_b|}{\tau_c} - 1 \right) & \text{for } |\tau_b| \geq \tau_c \\ 0 & \text{for } |\tau_b| < \tau_c \end{cases}, \quad (9)$$

where m_e is an empirical erosion rate, τ_c is the critical bottom shear stress of erosion, $\tau_b = \rho \nu \left[\partial u_1 / \partial x_3 \quad \partial u_2 / \partial x_3 \right] \Big|_{x_3=0}$ is the bottom shear stress and $|\tau_b|$ denotes its magnitude. Based on $|\tau_b|$, the friction velocity is calculated as $u_* = \sqrt{|\tau_b| / \rho}$. According to equation (9), the erosional flux is a function of time and space.

2.3 Implementation

A new numerical modeling system is built based on the theoretical formulation described in section 2.2. This section provides a brief overview of the numerical schemes and more details are given in Yue et al. (2019). The governing equations (2) and (6) are advanced in time sequentially by a third-order low-storage Runge-Kutta scheme (Williamson, 1980) and the Courant-Friedrichs-Lowy (CFL) condition is implemented to limit the size of a time step with a maximum CFL number of 0.5. Applying the pseudo-spectral scheme following Cortese and Balachandar (1995), the momentum equations (2) of the carrier flow phase are numerically solved with the corresponding boundary conditions (equations (4) and (5)). During each of the three time levels in a computational step, the standard two-stage (predictor and corrector) projection method (Chorin, 1968) is utilized to enforce a divergence-free velocity field of the carrier flow. Right after the velocity-projection for the carrier flow phase, the sediment volumetric concentration is computed by solving equation (6) in a way similar to the predictor stage of carrier flow with the boundary conditions (7) and (8).

The computational domain (Figure 1) is spatially discretized with a grid number of $N_1 \times N_2 \times N_3$ in the downslope, cross-slope and bed-normal direction, respectively. In the horizontal directions, the grid spacings are uniform and the discrete Fourier transform (DFT) is implemented in these two directions, which enforces the corresponding periodic boundary conditions. On the contrary, grid spacing is non-uniform in bed-normal direction and the Chebyshev-Gauss-Lobatto points are used. For the advection terms

296 in equations (2) and (6), the Arakawa scheme (Arakawa & Lamb, 1981) is utilized and
 297 the classical 3/2 rule is applied to remove the aliasing errors (Canuto et al., 1988). The
 298 semi-implicit second-order Crank-Nicolson method is implemented for the diffusion terms.
 299 After the application of the matrix multiplication technique (Peyret, 2002), the govern-
 300 ing equations are represented by a set of Helmholtz equations in wavenumber domain,
 301 which are directly solved by using the matrix-diagonalization method (Peyret, 2002).

302 Functioned by the Message Passing Interface (MPI) technique, the modeling sys-
 303 tem implements the two-dimensional (2D) pencil decomposition of the computational
 304 domain (Pekurovsky, 2012) in the horizontal directions for parallel computing. Through
 305 several standard benchmark tests, the newly build modeling system used in this study
 306 is carefully verified in Yue et al. (2019).

307 2.4 Experiments

308 Numerical simulations with a free-stream velocity amplitude of $U_w = 0.56 \text{ m/s}$
 309 and a wave period of $T_w = 10 \text{ s}$ are carried out in this study. This corresponds to a Stokes
 310 boundary layer thickness of $\Delta = \sqrt{2\nu/\Omega_w} = 1.8 \times 10^{-3} \text{ m}$ and the resulting Stokes
 311 Reynolds number $\text{Re}_\Delta = U_w \Delta / \nu = 1000$. According to earlier studies on transitional
 312 turbulence in WBBL (Jensen et al., 1989; Vittori & Verzicco, 1998), a WBBL with $\text{Re}_\Delta =$
 313 1000 is the intermittently turbulent regime, where the flow is turbulent only in part of
 314 a wave cycle. Following Ozdemir et al. (2010), we specify a constant settling velocity of
 315 sediment of $w = 5.0 \times 10^{-4} \text{ m/s}$ and the flocculation process (Soulsby et al., 2013) is
 316 ignored. Both the wave condition and settling velocity are similar to the energetic WSGF
 317 events observed in Eel River Shelf (Traykovski et al., 2000). The erodible bed has a crit-
 318 ical bottom shear stress of erosion of $\tau_c = 0.025 \text{ Pa}$ and an empirical erosion rate of $m_e =$
 319 $3 \times 10^{-7} \text{ m/s}$. With the given wave condition and settling velocity, these bed erodibil-
 320 ity parameters allow for a suspended sediment load close to the flow carrying capacity
 321 (Cheng et al., 2015).

322 In order to reach our research objectives (section 1), three different numerical ex-
 323 periments are taken in this study with a summary of them listed in Table 1. Initialized
 324 with the flow fields from the corresponding quasi-steady clear-flow runs (Yue et al., 2019),
 325 simulations start with zero sediment concentration in the computational domain. For
 326 the generation of downslope gravity flows, we specify a gentle bottom slope of 0.005 in

327 Case 1 and Case 2, which is in range of the commonly observed values on the active mar-
 328 gin of continental shelves. Moreover, wave direction is regarded to be an uncertainty in
 329 WSGF by present study. It is generally believed that during WSGF events, wave direc-
 330 tion is primarily parallel to the shelf slope (Scully et al., 2002; Wright et al., 2001; Wright
 331 & Friedrichs, 2006). However, since the local wave direction also depends on other fac-
 332 tors, such as wind direction and bathymetry, the corresponding WSGF dynamics due
 333 to wave direction need to be understood. Here, we quantify the variability due to wave
 334 direction by carrying two comparative runs between the Case 1 and Case 2, where the
 335 waves are specified to be parallel and perpendicular to the downslope direction, respec-
 336 tively (Table 1). For the purpose of comparison, the simulation of Case 0 is also taken
 337 where the wave direction is in the x_1 direction and the bottom slope is set to be zero.

338 Field data suggest that once a WSGF is generated, it requires constant energy in-
 339 put from waves to maintain sediment suspension in the WBBL and the WSGF no longer
 340 exists when the wave motion is too weak (Hale & Ogston, 2015; Traykovski et al., 2000).
 341 Therefore, the termination of WSGF, particularly regarding the requirement of sustain-
 342 ing wave motion as reported in the field observations, has to be reproduced by the present
 343 numerical simulation. Simulations of Case 1 and Case 2 are continued for another 20 wave
 344 periods from time $t_0 = 60T_w$ but with a damped wave forcing, which is written as

$$345 \quad S_i = U_w [\Omega_w \cos(\varphi) + \gamma \sin(\varphi)] \exp[\gamma(t - t_0)] m_i, \quad (10)$$

346 where the parameter γ is the damping rate. Note that the termination of WSGF in field
 347 during the waning stage of a storm could be more complicated than that described by
 348 equation (10).

349 For numerical simulations of WSGF presented in this study (Case 1 and Case 2),
 350 the computational domain has a size of $60\Delta \times 60\Delta \times 60\Delta$, which is confirmed to be
 351 large enough by computing the two-point correlation functions (Kim et al., 1987). This
 352 domain size is among the largest in the literature of simulating WBBL at Stokes Reynolds
 353 number $Re_\Delta = 1000$ (Vittori & Verzicco, 1998). Our choice of domain size ensures that
 354 the largest turbulent eddy in an oscillatory boundary layer is contained in the compu-
 355 tational domain. The domain is discretized with $256 \times 256 \times 193$ grid points in the two
 356 horizontal and bed-normal directions, respectively. The resulting grid resolution has been
 357 demonstrated to be fine enough for the study of fine sediment transport in the WBBL
 358 (Cheng et al., 2015; Ozdemir et al., 2010). As we will discuss later in section 3, the pres-

359 ence of the weak downslope gravity current has little effect on the strength of flow tur-
 360 bulence. Moreover, based on the peak friction velocity, this grid yields a resolution in
 361 wall unit of $\delta x_1^+ = \delta x_2^+ = 11.8$. In the bed-normal direction, we obtain $\delta x_3^+ = 0.2$
 362 close to the wall and $\delta x_3^+ = 24.8$ in the middle of the water column. This grid resolu-
 363 tion is similar to the one used in Ozdemir et al. (2010).

364 **2.5 Variable Decomposition and Notation**

365 The problem investigated in this study involves turbulent flow generated by wave
 366 motions and the wave-averaged current driven by downslope gravity. We adopt the triple
 367 decomposition method (Reynolds & Hussain, 1972) to isolate the weak downslope grav-
 368 ity current and the organized variations in the turbulent fluctuating flow field. The triple
 369 decomposition is applied in a similar manner as other turbulence-resolving numerical stud-
 370 ies for a current-wave-fluctuation decomposition (Nelson & Fringer, 2018). We decom-
 371 pose a variable ψ into a current component $\langle \psi \rangle_c$, a wave component $\langle \psi \rangle_w$ and a fluctu-
 372 ating component ψ' as follows

373
$$\psi = \langle \psi \rangle_c (x_3) + \langle \psi \rangle_w (x_3; t) + \psi' (x_1, x_2, x_3; t). \quad (11)$$

374 To calculate each component shown in equation (11), we define the time- and phase-
 375 averaged components of a variable $\psi (x_1, x_2, x_3; t)$ respectively as

376
$$\bar{\psi} = \frac{1}{MT_w} \int_{M_1 T_w}^{M_2 T_w} \psi (x_1, x_2, x_3; t) dt, \quad (12)$$

377
$$\langle \psi \rangle_p = \frac{1}{M} \sum_{n=0}^{M-1} \psi (x_1, x_2, x_3; t + nT_w), \quad (13)$$

378 where $M = M_2 - M_1$ is the number of wave periods in time range $[M_1 T_w \quad M_2 T_w]$
 379 during which the data analysis is taken. Corresponding to the definition of time aver-
 380 age (equation (12)), the average operation over a spatial dimension is written as

381
$$\langle \psi \rangle_i = \frac{1}{L_i} \int_0^{L_i} \psi (x_1, x_2, x_3; t) dx_i. \quad (14)$$

382 Consequently, we approximate the current component $\langle \psi \rangle_c$ by the time- and plane-averaging
 383 of the variable, namely

384
$$\langle \psi \rangle_c = \langle \bar{\psi} \rangle_{12}. \quad (15)$$

385 The wave component $\langle \psi \rangle_w$ is then extracted by subtracting the current component $\langle \psi \rangle_c$
 386 from the phase- and plane-averaged quantity $\langle \psi \rangle_{p12}$ as

$$387 \quad \langle \psi \rangle_w = \langle \psi \rangle_{p12} - \langle \psi \rangle_c. \quad (16)$$

388 After obtaining the wave and current components, the turbulent fluctuating component
 389 ψ' is computed as

$$390 \quad \psi' = \psi - \langle \psi \rangle_c - \langle \psi \rangle_w = \psi - \langle \psi \rangle_{p12}. \quad (17)$$

391 The notation defined here is particularly useful to present the statistically-averaged
 392 quantities. Note that the average operations defined in equations (12) to (14) are com-
 393 mutative. As a result, the combination of multiple average operations can be denoted
 394 by the subscripts of the average operator ' $\langle \rangle$ ', except for the time average. Accordingly,
 395 the turbulence kinetic energy (TKE) is defined as $k = \langle u'_i u'_i \rangle_{p12} / 2$ and the expression
 396 $\Phi = \langle \phi \rangle_{123}$ represents the domain-averaged sediment concentration. We denote $c =$
 397 $(1 + R) \rho \phi$ as the sediment mass concentration and $F_g = \langle f_g \rangle_{123}$ as the domain-averaged
 398 mass transport rate of sediment in the downslope direction, where $f_g = c u_1$ is the downs-
 399 slope sediment mass flux. Then, the ratio of sediment transport rate to the domain-averaged
 400 mass concentration is used to quantify the bulk gravity current speed, which reads

$$401 \quad U_{gf} = \frac{F_g}{\langle c \rangle_{123}}. \quad (18)$$

402 3 Results

403 3.1 Intra-wave Evolution of Near-bed Dynamics

404 3.1.1 Bottom Shear Stress and Suspended Sediment

405 The temporal evolution of the domain-averaged sediment concentration and the
 406 downslope current speed illustrates the formation and development of WSGF (Figure
 407 2). As shown in Figure 2(b), for all the three cases, the domain-averaged sediment con-
 408 centrations start to increase rapidly within the first ten waves and the equilibrium val-
 409 ues are attained at about the 20th wave period. As more sediments are suspended into
 410 the domain, the sediment depositional (downward) flux near the bed also increases. In
 411 the incipient stage, the plane- and wave-averaged net erosion-deposition rate ($q_n = q_e +$
 412 q_d) of sediment at the bed is of order 10^{-5} m/s, which drops to 10^{-8} m/s after 20 wave
 413 periods. In other words, the averaged depositional flux balances with the erosional flux

414 after the incipient stage and the amount of sediment in suspension quantified by the domain-
 415 averaged sediment concentration is in equilibrium. Wave direction shows negligible effect
 416 on the domain-averaged sediment concentration at equilibrium, although slightly lower
 417 Φ is found for Case 0 in which there is no bottom slope.

418 For cases with a mild bottom slope of 0.005, the suspended sediments drive downslope
 419 gravity currents and their temporal evolutions are illustrated by carrying out time-
 420 average of the bulk gravity current speed (equation (18)) over every ten wave periods
 421 (Figure 2(c)). In Case 2, where the waves are perpendicular to the bottom slope, the de-
 422 velopment of the downslope gravity current speed directly correlates with the amount
 423 of sediment in suspension (compare Figure 2(b) and (c)). During the incipient stage, as
 424 more sediments are suspended over the sloping bed, they start to drive a downslope cur-
 425 rent whose speed reaches its equilibrium value of around 1.66 cm/s (Table 2) after about
 426 30 wave periods. When waves are in the same direction with respect to the bottom slope
 427 (Case 1), we observe a notably weaker downslope gravity current, reaching about 0.93 cm/s
 428 after 40 wave periods.

429 Figure 3 shows the time evolution of the free-stream velocity, the phase-plane-averaged
 430 magnitude of bottom shear stress $|\langle \tau_b \rangle_{p12}|$ and phase-domain-averaged sediment concen-
 431 tration $\langle \phi \rangle_{p123}$ (phase-averaging over the 40th to 60th waves, see Table 2). This figure
 432 reveals that, all three cases show similar intra-wave evolution of bottom shear stress, sug-
 433 gesting that the effects of mild bottom slope and wave direction are small. In each of the
 434 cases, there is an almost continuous erosion of sediment (upward erosional flux) since the
 435 bottom shear stress magnitude is greater than the critical shear stress of erosion of 0.025 Pa
 436 in more than 96% of a wave period (Figure 3(b)). These bottom shear stress time se-
 437 ries are contrasted with the analytical solution of laminar flow (Jensen et al., 1989) in
 438 order to illustrate the intermittently turbulent flow features. During the first accelera-
 439 tion stage between $\varphi = 0$ and $2\pi/6$, bottom shear stresses of the three cases are close
 440 to the laminar solution and in particular, the well-known phase lead of $\pi/4$ (Cheng et
 441 al., 2015) is matched. Immediately after $\varphi = 2\pi/6$, the bottom shear stresses deviate
 442 from the laminar solution and increase rapidly to reach their peak values of about 0.85 Pa
 443 at around the wave crest of $\varphi = 3\pi/6$. During the deceleration stage between $\varphi = 3\pi/6$
 444 and $6\pi/6$, the bottom shear stresses decrease quickly to zero before the flow reversal ($\varphi =$
 445 $6\pi/6$). The intra-wave evolution of bottom shear stress observed here for cases with a

446 mild slope are typical of transitionally turbulent WBBL flow at this Reynolds number
 447 (Vittori & Verzicco, 1998; Ozdemir et al., 2010).

448 In contrast to the bottom shear stresses, the domain-averaged sediment concen-
 449 trations show much lower temporal variability throughout the wave cycle (Figure 3(c)).
 450 The ratio of the sediment settling velocity to the bed friction velocity is used to quan-
 451 tify the importance of settling effect versus turbulent suspension. Based on the averaged
 452 amplitude of bottom shear stress $\langle |\tau_b| \rangle_c = 0.43 \text{ Pa}$ (Table 2), the averaged bed friction
 453 velocity is calculated as 2.07 cm/s . The fact that the settling velocity of sediment used
 454 in this study is only $5 \times 10^{-4} \text{ m/s}$, the ratio of settling velocity to friction velocity has
 455 an averaged value of 2.41×10^{-2} , which is much smaller than unity. In other words, the
 456 turbulent suspension dominates the settling effect of sediment. Thus, similar amount of
 457 sediment is suspended in the computational domain without significant temporal change.
 458 Nevertheless, a notable increase of sediment concentration is still observed around the
 459 burst of bottom shear stress. We also find that adding a small bottom slope only slightly
 460 increases the bottom shear stress and thus the suspended sediment load (by about 3%),
 461 while the effect of wave direction on domain-averaged sediment concentration is almost
 462 negligible (Figure 3(c)).

463 3.1.2 Transitionally Turbulent Feature of WSGF

464 The present turbulence-resolving simulation results provide an opportunity to ex-
 465 amine the transitionally turbulent feature of WSGF, through the visualization of coher-
 466 ent turbulent structures during the acceleration (Figure 4) and deceleration (Figure 5)
 467 instants. Using Case 2 as an example, we apply the criterion of swirling strength λ_{ci} (Zhou
 468 et al., 1999), which represents the local fluid rotation speed, to visualize coherent tur-
 469 bulent structures. At $\varphi = 0$ and $\pi/6$, larger but weaker coherent turbulent structures
 470 are elevated from the bed. Interestingly, isosurface of λ_{ci} at $\varphi = 2\pi/6$ shows very high
 471 spatial variability with a portion of the domain ($x_1 = 0 \sim 40\Delta$) almost having no λ_{ci}
 472 exceeding the corresponding isovalue compared to other portion of the domain ($x_1 =$
 473 $40\Delta \sim 60\Delta$). On the other hand, much more intense coherent turbulent structures at
 474 $\varphi = 3\pi/6$ and $4\pi/6$ are of smaller size and very densely populated close to the bed. The
 475 features of coherent turbulent structure (and intensity) are distinctly different between
 476 just a short time interval of $\varphi = 2\pi/6$ and $3\pi/6$, which is consistent with the time se-
 477 ries of bottom shear stress discussed in Figure 3(b). Moreover, consistent with the fine

478 sediment assumption (section 2.2.1), the isosurfaces of sediment concentration generally
 479 respond directly to the coherent turbulent structures throughout a wave cycle, which is
 480 especially evident at $\varphi = 2\pi/6$.

481 Figure 6 presents the plane- and phase-averaged profiles of the streamwise flow ve-
 482 locity, the suspended sediment concentration and the turbulence kinetic energy at phase
 483 of the lowest ($\varphi = \pi/6$) and the highest ($\varphi = 3\pi/6$) turbulence intensity for Case 2.
 484 Although the magnitude of TKE intensity increases evidently from $\varphi = \pi/6$ to $\varphi =$
 485 $3\pi/6$ (Figure 6(c,f)), the sediment concentration only increases slightly (Figure 6(b,e)).
 486 More importantly, both concentration profiles show the feature of a sharp negative sed-
 487 iment concentration gradient, called the lutocline, located around $x_3 = 15.5\Delta$. As dis-
 488 cussed in previous studies (Cheng et al., 2015; Ozdemir et al., 2010), the formation of
 489 lutoclines is a prominent feature resulting from the sediment-induced stable density strat-
 490 ification, which attenuates fluid turbulence. Consequently, a remarkable amount of sus-
 491 pended sediment load is persistently confined between $x_3 = 0$ and $x_3 = 20\Delta$ (about
 492 3.6 cm), having sufficient buoyancy anomaly to further drive a downslope gravity flow
 493 (section 3.2). From the intensity of TKE, the transitional characteristics of flow discussed
 494 in Figures 4 and 5 are confirmed. At $\varphi = \pi/6$, the turbulence is more than 10 times
 495 weaker than that under the wave crest at $\varphi = 3\pi/6$, implying an evident change in level
 496 of turbulence during the intra-wave evolution.

497 3.2 Gravity Currents on Gentle Bottom Slope

498 The intra-wave results presented in section 3.1 confirm that the transitionally tur-
 499 bulent WBBL has a two-layer like structure and persistently suspends sediment within
 500 a few centimeters above the bed throughout a wave cycle. When a gentle bottom slope
 501 presents in the sediment-laden WBBL (Case 1 and Case 2), the resulting downslope grav-
 502 itational force caused by the near-bed density anomaly from sediment suspension can
 503 drive an offshore-directed gravity flow. This important gravity-flow process is presented
 504 in this section, which relates to the research objective 2).

505 3.2.1 Downslope Current and Sediment Transport

506 To facilitate the comparison with other field and model studies, we present model
 507 results in this section using sediment mass concentration c . The time- and plane-averaged

(over the last 20 wave periods, see Table 2) vertical profiles of the sediment mass concentration $\langle c \rangle_c$, the downslope current velocity $\langle u_1 \rangle_c$, the downslope sediment mass flux $\langle f_g \rangle_c$ and the TKE (\bar{k}) for the three cases are shown in Figure 7. Averaged sediment concentration profiles are similar for all three cases, particularly regarding the formation of the lutocline and the two-layer like structure (Figure 7(a)). The close similarity in concentration profiles is consistent with the almost identical turbulence kinetic energy profiles shown in Figure 7(d), since sediments are primarily suspended by turbulence. Moreover, sediments in suspension are constrained in a layer close to the bottom. To be specific, more than 91% of the suspended sediments are below the lutocline (defined as the inflection point of sediment concentration profile) which is located at $x_3 = 2.82$ cm, while the height of the computational domain is $L_3 = 10.70$ cm. Right above the bed, the mass concentration of sediment reaches about 26 kg/m^3 (g/L). This amount of suspended load indeed drive an offshore-directed gravity current over the gentle bottom slope of 0.005 specified here (Figure 7(b)). In both Case 1 and Case 2, the mean current profiles increase from zero at the bed to their peak values near the location of lutocline. Moving upward, the currents decrease slowly to the top of the computational domain. The offshore currents, along with the suspended sediment, cause the corresponding offshore sediment fluxes, whose profiles have their maximums located in the middle of the sediment layer (Figure 7(c)). Hence, it is clear that these offshore currents are associated with the near-bed suspended sediment load.

One important feature to be noted is that, while the sediment concentration and turbulence kinetic energy profiles are nearly identical in Case 1 and Case 2, the resulting intensity of downslope gravity current and thus the sediment transport are clearly dependent on the wave direction. Specifically, for Case 1 with waves parallel to the downslope direction, the offshore directed current is weaker and has a peak downslope velocity of 1.2 cm/s. This value increases to 2.1 cm/s in Case 2 when waves are perpendicular to the slope direction. In addition, the maximum of offshore sediment mass flux of Case 2 is larger than that of Case 1 by a factor of 1.57.

Although WSGFs require constant support from wave-induced fluid turbulence in WBBL and thus are considered fundamentally different from typical turbidity currents (Parsons et al., 2009), there are still some similarities between these two sediment-driven gravity flows. Similar to turbidity current, the vertical structure of the mean velocity profiles in WSGFs obtained here consists of two regions. As expected for the conven-

541 tional turbulent boundary layer, there is an inner region approximately below the luto-
 542 cline where the gradient of velocity is positive (Figure 7(b)). This region contains the
 543 majority of sediment load (Figure 7(a)) where more than 90% of the sediment transport
 544 occurs in the present cases (Figure 7(c)). In contrast, an outer region is defined above
 545 the inner region where the velocity gradient is negative. Clearly from Figure 7(b), the
 546 lutocline effectively separates the inner and outer regions. Moreover, above the lutocline
 547 in the outer region, the profiles of TKE are close to zero which are also similar to typ-
 548 ical turbidity currents (Meiburg & Kneller, 2010). In the present WSGF simulations, the
 549 time-averaged TKE at lutocline is only about 3% of its maximum in the inner region (Fig-
 550 ure 7(d)).

551 3.2.2 Bottom Slope and Wave Boundary Layer

552 A key finding drawn from section 3.2.1 is that, by adding a gentle bottom slope of
 553 0.005 to sediment-laden WBBL (relative to Case 0), a weak cross-shelf downslope cur-
 554 rent is generated. Although the wave direction relative to bottom slope plays an impor-
 555 tant role in determining the intensity of resulting downslope gravity current (Figure 7(b)),
 556 the suspended sediment load and turbulence intensity are both insensitive to the exis-
 557 tence of bottom slope and thus the downslope current (Figure 7(a,d)).

558 The effect of the bottom slope on the modeled suspended sediment load is explained
 559 by examining the balance of sediment flux budget in bed-normal direction (equation (A1)
 560 in Appendix A), which consists of the turbulent suspension flux ($\langle u'_3 \phi' \rangle_c$), the settling
 561 flux ($-w \langle \phi \rangle_c$), and the diffusive/molecular sediment flux ($-K \partial \langle \phi \rangle_c / \partial x_3$). The flux bud-
 562 gets are computed here for all three cases, which are all very close to each other (Fig-
 563 ure 8). In the region very near the bed ($x_3 < 0.5\Delta$), the flow is laminar and the up-
 564 ward diffusive flux from molecular diffusivity is responsible for suspending sediments. In
 565 the layer above the viscous sublayer ($x_3 > 1\Delta$) and below the lutocline ($x_3 < 16\Delta$),
 566 the settling flux is mainly balanced by the turbulent suspension flux, suggesting that the
 567 majority of sediment load in WSGF is maintained by the fluid turbulence in the WBBL.
 568 Thus, the similar vertical distribution of sediment mass concentration (Figure 7(a)) is
 569 consistent with the almost identical distribution of turbulence intensity (Figure 7(d)) for
 570 the three cases. Hence, we conclude that the similar concentration profile is mainly driven
 571 by the same and relatively large oscillatory velocity amplitude U_w ($U_w / \bar{U}_{gf} > 30$, see
 572 Table 2), while the presence of a mild bottom slope and the difference in wave direction

573 have a minor effect. Approaching the lutocline, the diffusive flux begins to increase again
 574 around $x_3 = 10\Delta$, which is the result of flow turbulence attenuation by the sediment-
 575 induced stable density stratification. The local maximum of diffusive flux intercepts with
 576 the corresponding turbulent suspension flux at the location of lutocline, above which the
 577 molecular diffusive flux becomes dominant again in the flux budget. The existence of a
 578 local maximum in the molecular diffusive flux around the lutocline is regarded as a di-
 579 rect evidence of the two-layer like fine sediment transport system.

580 The small effect of bottom slope on the intensity of flow turbulence is explained
 581 by the time-averaged TKE budget of \bar{k} (equation (A2) in Appendix A). The production
 582 term in the budget consists of three components, which read $\langle \mathcal{P} \rangle_c = \langle \mathcal{P}_c \rangle_c + \langle \mathcal{P}_w \rangle_c +$
 583 $\langle \mathcal{P}_b \rangle_c$, corresponding to the production (or destruction) of \bar{k} respectively from the mean
 584 current, waves and buoyancy. From Figure 9, it is evident that in all three cases, the dom-
 585 inant terms in the time-averaged TKE budget are the turbulence dissipation rate and
 586 the wave production. They balance with each other, except very near the bed, where tur-
 587 bulence production must decay to zero at the bed. The other two sources, namely the
 588 mean current and the buoyancy productions, contribute very little to the time-averaged
 589 TKE budget and are nearly three orders of magnitude smaller than the wave produc-
 590 tion (see the inset of Figure 9). In other words, the WBBL gets most of the turbulence
 591 production from wave motion, which keeps the sediment load in suspension. Hence, the
 592 downslope gravity currents obtained in Case 1 and Case 2 are clearly wave-supported.
 593 Since the dominant terms in the energy budget ($\langle \varepsilon \rangle_c$ and $\langle \mathcal{P}_w \rangle_c$) are similar for all three
 594 cases, it is straightforward to understand that the resulting time-averaged TKE profiles
 595 shown in Figure 7(d) are also similar. Moreover, the buoyancy production ($\langle \mathcal{P}_b \rangle_c$) in all
 596 three cases is negative, because the sediment-induced stable stratification dominates $\langle \mathcal{P}_b \rangle_c$,
 597 which attenuates flow turbulence. This implies that the simulated WSGFs are not self-
 598 sustaining. The profiles of $\langle \mathcal{P}_b \rangle_c$ in these cases are all close to each other due to the sim-
 599 ilar suspended sediment loads (Figure 7(a)). There is a notable difference in production
 600 due to mean current ($\langle \mathcal{P}_c \rangle_c$), because the wave direction affects the current velocity in
 601 WSGF. However, the magnitude of $\langle \mathcal{P}_c \rangle_c$ is very small when compared to that of $\langle \mathcal{P}_w \rangle_c$
 602 and it has negligible effect on the overall plane- and time-averaged TKE budget.

603 **3.3 WSGF Mechanics and the Role of Waves**604 **3.3.1 Cross-shelf Current Driven by Downslope Gravity**

605 As shown in section 3.2, the presence of a gentle bottom slope leads to downslope
 606 gravity currents with a magnitude of a few cm/s. Importantly, the magnitude of the downs-
 607 slope gravity current is dependent on the wave direction. The reasoning process of the
 608 effect of wave direction begins with the momentum flux balance in the bed-normal di-
 609 rection (see the derivation of equation (A8) in Appendix A). Integrating equation (A8)
 610 again from 0 to x_3 and applying the no-slip bottom boundary condition, we obtain an
 611 explicit expression of the downslope current

$$612 \quad \langle u_1 \rangle_c = \frac{1}{\nu} \left(\int_0^{x_3} \langle u'_1 u'_3 \rangle_c dx'' + Rgn_1 \int_0^{x_3} \int_{x''}^{L_3} \langle \phi \rangle_c dx' dx'' \right), \quad (19)$$

613 where x' and x'' are dummy variables. From equation (19), it is clear that the downs-
 614 slope current is determined by the vertically integrated time-averaged Reynolds stress (first
 615 term on RHS) and the strength of downslope driving force (last term on RHS). As it has
 616 been shown in previous sections that for Case 1 and Case 2, the concentration profiles
 617 are similar, regardless of wave direction. Therefore, equation (19) essentially indicates
 618 that the wave direction must affect the intensity of downslope gravity current through
 619 modifying the time-averaged Reynolds shear stress $\langle u'_1 u'_3 \rangle_c$.

620 As shown in Figure 10(a) for the balance of momentum flux (equation (A8)), the
 621 vertical profiles due to downslope buoyancy flux for the two cases are almost identical
 622 and the downslope driving forces associated with wave direction are very similar. On the
 623 other hand, we observe a notable difference in the vertical profiles of the time-averaged
 624 Reynolds shear stress $\langle u'_1 u'_3 \rangle_c$. When wave direction is parallel to the slope (and downs-
 625 slope gravity current), there exists a stronger time-averaged Reynolds shear stress to bal-
 626 ance the downslope buoyancy flux. Therefore, the resulting net momentum flux on the
 627 LHS of equation (A8) becomes smaller. Figure 10(b) shows the net momentum fluxes
 628 and it is evident that for Case 1 with waves parallel to the downslope current, the net
 629 momentum flux is about 50% smaller than that of Case 2. This explains why the result-
 630 ing WSGF speed for Case 1 is about factor two smaller (Table 2). Note from Figure 10
 631 that the momentum fluxes associated with each term on the RHS of equation (A8) are
 632 about a factor eight greater than the net momentum flux. Therefore, we realize that a
 633 10% difference in the time-averaged Reynolds shear stress $\langle u'_1 u'_3 \rangle_c$ due to wave direction

634 can lead to almost a factor two difference in the net momentum flux and hence the re-
 635 sulting downslope currents.

636 The dynamics of WSGF represented by the present simulations is not straightforward.
 637 Essentially, one needs to recognize that WSGF is a “small” magnitude time-averaged
 638 gravity current (a few cm/s) generated by suspended sediments, which are sustained by
 639 “large” magnitude of wave velocity (about 50 cm/s). Therefore, with the present high
 640 resolution numerical simulations for different scenarios, we can extract the difference and
 641 similarity in the momentum and turbulence energy balances to gain insight into WSGF
 642 dynamics.

643 *3.3.2 Dependence on Wave-induced Fluid Turbulence*

644 According to the analysis of time-averaged TKE budget equation in section 3.2.2,
 645 the WSGFs obtained from the present simulations are sustained by persistent wave mo-
 646 tions. To intuitively demonstrate this important feature, simulations of Case 1 and Case
 647 2 are continued for another 20 wave periods from time $t_0 = 60T_w$ with a damped wave
 648 forcing (equation (10)). In this study, we specify $\gamma = -0.044$ Hz in order to obtain a
 649 sufficient but gradual decay of the wave motions within about 6 wave periods (see Fig-
 650 ure 11(a)). It is evident that the fluid turbulence responds rapidly with respect to the
 651 wave motion. The instantaneous plane-averaged TKE profiles indicate that turbulence
 652 is almost completely dissipated within first three damped waves (Figure 11(d)). The domain-
 653 averaged sediment concentrations show a milder decay than plane-averaged TKE, although
 654 almost all the sediments are deposited within 15 wave periods after the wave forcing has
 655 been attenuated. To be specific, at $t - t_0 = 5.25T_w$, more than 30% of sediment is still
 656 suspended in the computational domain and it is until $t - t_0 = 15T_w$, the domain loses
 657 most (99%) of the suspended sediment (Figure 11(b)). The longer retention time of sus-
 658 pended sediment in the computational domain is explained by the small settling veloc-
 659 ity of sediment (or small fall parameter) used in these simulations. Due to the decay of
 660 suspended sediment load in the domain, the corresponding downslope sediment fluxes
 661 F_g also decrease accordingly (Figure 11(c)). This is most clear by inspecting the F_g of
 662 Case 2, in which the downslope sediment flux is perpendicular to the wave motion. The
 663 downslope sediment flux decays as fast as the concentration and the WSGF is terminated
 664 within $15T_w$.

665 **4 Discussion**666 **4.1 Comparison with observations**

667 Clearly from section 3.2, adding a gentle bottom slope to sediment-laden WBBL
 668 leads to WSGFs. For Case 1, in which the wave direction is parallel to the downslope
 669 direction, we obtain the peak downslope velocity of 1.2 cm/s near the location of luto-
 670 cline. This value is about 50% larger than that reported by Ozdemir (2016) of 0.8 cm/s,
 671 which uses a prescribed suspended sediment load near flow carrying capacity. This sug-
 672 gests that WSGF dynamics are dependent on the exchange of sediment with the bed.
 673 However, the resulting cross-shelf current speed is still smaller than most of the field ob-
 674 served data. For instance, through extrapolation of velocity to the WBBL, Traykovski
 675 et al. (2000) suggested a downslope current speed of about 10 cm/s. In later field obser-
 676 vations, collected at the Po prodelta, the peak downslope velocity of an energetic WSGF
 677 event was reported to be around 5 cm/s (Traykovski et al., 2007). Jaramillo et al. (2009)
 678 reported $3 \sim 5$ cm/s for WSGF events observed at Atchafalaya shelf. Through indirect
 679 estimate of the sediment flux budget, Hale and Ogston (2015) obtained lower values of
 680 WSGF velocity of $2 \sim 3$ cm/s on the continental shelf offshore of the Waipaoa River.
 681 However, the shelf slope in these later two field sites is milder (~ 0.003) than that re-
 682 ported by Traykovski et al. (2000). A recent measurement of the downslope current speed
 683 of WSGF by Flores et al. (2018) in a mixed sediment site also suggested a speed of 5 cm/s.
 684 A more careful comparison of our model results with these field data indicates that the
 685 main reason that the present simulations predict lower downslope gravity current speed
 686 is because the computed near-bed sediment mass concentration is only about 26 kg/m^3
 687 (g/L). This mass concentration is about a factor two smaller than field-reported values
 688 during WSGF events. According to Cheng et al. (2015), further reducing the critical shear
 689 stress of erosion to increase the suspended sediment load while keeping all the other pa-
 690 rameters the same will cause flow laminarization. In other words, for the present wave
 691 intensity and the flat bed setup, WBBL is transitionally turbulent and the suspended
 692 sediment load cannot be further enhanced beyond its sediment carrying capacity.

693 We have investigated the effect of wave direction on WSGF. Results suggest the
 694 wave direction plays an important role in determining the intensity of resulting downs-
 695 slope gravity current, although the sediment load (Figure 7(a)) and turbulence intensity
 696 (Figure 7(d)) are both insensitive to the wave direction. As a result, having waves per-

697 perpendicular to the downslope direction only increases the peak downslope gravity current
 698 velocity to 2.1 cm/s, which is still on the low side when compared to observed flow ve-
 699 locities. Overall, our simulation results imply that other factors, such as the presence
 700 of small bedforms (Traykovski et al., 2015) and ambient currents (Ma et al., 2008), should
 701 be investigated in the future to understand their roles in enhancing the suspended sed-
 702 iment load and the resulting downslope current speed. Moreover, the present study ig-
 703 nores the flocculation process and the hindered settling of fine sediment. We simply spec-
 704 ify a constant settling velocity of 0.5 mm/s, which is comparable to commonly accepted
 705 value for fine sediment (Hill et al., 2000). The effects of flocculation and hindered set-
 706 tling on the settling velocity in WBBL and the resulting WSGF dynamics also warrant
 707 future investigation. Finally, the present study assumes a perfect symmetric wave mo-
 708 tion while in reality, wave velocity skewness may also play a role.

709 4.2 Parameterization of WSGFs

710 The parameterization of WSGFs is necessary in quantifying and predicting the cross-
 711 shelf fine sediment transport (Scully et al., 2003; Wright et al., 2001). One of the most
 712 common parameterization methods is formulated through the balance between the wave-
 713 averaged bottom friction and the sediment-induced downslope gravitational force. The
 714 momentum balance in downslope direction presented in equation (A7) is completely con-
 715 sistent with this classical force balance, commonly called the Chezy equation (Wright
 716 et al., 2001)

$$717 B \sin \theta = C_D U_g U_{max}, \quad (20)$$

718 where C_D is a non-dimensional bottom drag coefficient, U_g is depth-averaged downslope
 719 velocity, U_{max} is the magnitude of velocity at top of the bottom boundary layer. The
 720 depth-integrated buoyancy anomaly is represented by B , which reads

$$721 B = gR \int_0^{L_3} \phi dx_3. \quad (21)$$

722 The idea behind the parameterization equation (20) is that the averaged downslope ve-
 723 locity of gravity current U_g can be calculated using the depth-integrated buoyancy anomaly
 724 B and U_{max} (which can be estimated from field measured data), when a reasonable value
 725 of drag coefficient C_D is specified.

726 The Chezy formulation was originally developed for parameterizing auto-suspending
 727 turbidity currents and there was no ambiguity in estimating U_{max} using U_g (Meiburg

728 & Kneller, 2010). The auto-suspension criterion $\beta = w/U_g\theta$ proposed by Parker (1982)
 729 is calculated for WSGF here (see Table 3). Both cases show $\beta \gg 1$ and thus WSGFs
 730 are distinct from the auto-suspending turbidity currents. As a result, the parameter U_{max}
 731 is well-approximated by U_w rather than U_g , since $U_g \ll U_w$. Moreover, the present sim-
 732 ulation results indicate that the bottom drag coefficient is 0.0047 for waves that prop-
 733 agate in the cross-shelf direction (Case 1), see Table 3. This value is on the high end of
 734 C_D reported by the review paper of Wright and Friedrichs (2006), who suggested $C_D =$
 735 $0.002 \sim 0.005$. This value is also consistent with the value of $C_D = 0.006$ suggested
 736 by the recent turbulence-resolving numerical investigation of Ozdemir (2016). It should
 737 be noted that in most of the field studies, C_D is estimated to be around 0.003 (Hale &
 738 Ogston, 2015; Flores et al., 2018) or lower (Traykovski et al., 2000, 2007). Interestingly,
 739 by considering the direction of waves are in along-shelf direction (Case 2), we obtain sig-
 740 nificantly lower C_D of 0.0029. In other words, the nearly factor two variability of C_D re-
 741 ported in the field observation between $C_D = 0.003 \sim 0.005$ may be explained by the
 742 effect of wave direction. The enhanced drag coefficient when the waves are in the same
 743 direction of the downslope current is due to increased time-averaged Reynolds shear stress,
 744 as presented in Figure 10. Here, it is also useful to point out that this feature is consis-
 745 tent with the well-known “apparent roughness” concept (Grant & Madsen, 1986). This
 746 concept indicates that a more significant roughness (larger than the physical roughness
 747 of the bed) is experienced by the current when superimposed a wave motion, due to the
 748 enhanced turbulence in the WBBL. As discussed in detail by Grant and Madsen (1986),
 749 this enhanced roughness depends on wave direction and it is maximized when waves are
 750 in the same direction with the current.

751 Historically, there exists a discrepancy in the estimation of B for WSGF modeling
 752 (Parsons et al., 2009). A widely-used idea is to estimate the buoyancy anomaly based
 753 on velocity magnitude U_{max} (or U_w for our cases) and an empirical critical Richardson
 754 number Ri_{cr} as $B = Ri_{cr}U_{max}^2$. By assuming that the WBBL maintains its sediment-
 755 carrying capacity, Ri_{cr} is suggested to be 0.25 (Scully et al., 2002; Wright et al., 2001;
 756 Wright & Friedrichs, 2006). However, recent field measurements (Hale & Ogston, 2015;
 757 Traykovski et al., 2015), laboratory experiments (Lamb & Parsons, 2005) and numer-
 758 ical studies (Ozdemir, 2016) suggest that the Ri_{cr} of WSGF is much lower than the widely-
 759 used value of 0.25. As summarized in Table 3, the present simulations give an empiri-
 760 cal critical Richardson number Ri_{cr} around 0.01, regardless of wave direction. This value

761 is significantly smaller than 0.25, but it is consistent with recent field observations of Flores
 762 et al. (2018), who reported $Ri_{cr} = 0.01$ and laboratory experiments of Lamb and Par-
 763 sons (2005) showing $Ri_{cr} = 0.013$ where sediment concentration is directly measured.
 764 Although there exist larger uncertainties in earlier field measurements, many WSGF events
 765 are observed in the field to occur at Ri_{cr} much lower than 0.25 (Traykovski et al., 2007).
 766 It should be pointed out that a sediment-carrying capacity equivalent to $Ri_{cr} = 0.25$
 767 is often observed in fully turbulent flow, such as turbidity current or tidal boundary layer
 768 (J. H. Trowbridge & Kineke, 1994). Our numerical investigation indicates that for WSGF
 769 generated at Stokes Reynolds number around 1000, the resulting WBBL is transition-
 770 ally turbulent with a lower carrying capacity and the corresponding Ri_{cr} is one order of
 771 magnitude smaller than 0.25.

772 **5 Conclusions**

773 Turbulence-resolving numerical simulations of wave-supported sediment-driven grav-
 774 ity flows in energetic wave condition at a slope of 0.005 over a flat bed are reported in
 775 this study. By allowing sediments to be freely eroded from and deposited to the bottom,
 776 we address our research objectives by investigating the relationship between the wave
 777 boundary layer turbulence, the suspended sediment load and the resulting downslope grav-
 778 ity current in response to wave directions. Through the intra-wave evolution of bed shear
 779 stresses, coherent turbulent structures and sediment concentration and turbulence statis-
 780 tics, we confirm that the present problem belongs to the category of transitionally tur-
 781 bulent flow. While the transitionally turbulent wave bottom boundary layer can support
 782 a maximum sediment load approaching 30 kg/m^3 which leads to sufficient sediment-induced
 783 buoyancy anomaly to drive the downslope gravity flow, as far as the maximum sediment
 784 load and leading-order turbulence statistics are concerned, the effect of wave direction
 785 is negligible. The reason behind this finding is explained through the time- and plane-
 786 averaged sediment mass flux budget and the TKE budget. Simulation results reveal that
 787 the maximum downslope gravity current speed is around 2 cm/s when the wave direc-
 788 tion is perpendicular to the downslope direction. However, as the waves become paral-
 789 lel to the slope, the resulting downslope current intensity and hence the downslope sed-
 790 iment flux are reduced by a factor of two. From analyzing the time- and plane-averaged
 791 momentum flux balance, we conclude that the wave direction changes the time-averaged

792 Reynolds stress experienced by the downslope current. Hence, the mechanism is consis-
 793 tent with the classical apparent roughness concept.

794 Simulation results are further used to examine the drag coefficient C_D and the em-
 795 pirical critical Richardson number Ri_{cr} for the parameterization of WSGFs. The drag
 796 coefficient C_D is found to be around 0.005 for waves are in along-shelf direction while
 797 it reduces to about 0.003 when waves are in cross-shelf direction. Simulation results also
 798 indicate that for the present flat bed condition driven solely by the wave motions in a
 799 transitionally turbulent WBBL, the maximum sediment load is limited and the result-
 800 ing Ri_{cr} is significantly lower than 0.25. For some field observations reporting WSGF
 801 events occur at higher carrying capacity (and possibly more intense downslope gravity
 802 current), the present simulations imply that other processes, such as the presence of am-
 803 bient current and bedforms, may play important roles.

804 **Appendix A Derivation of Budget/Balance Equations**

805 We first discuss the mass balance equation for the sediment volumetric flux in bed-
 806 normal direction. After applying the triple decomposition (section 2.5) in equation (6)
 807 and taking the time- and plane-averaging on both sides, the following budget equation
 808 of sediment volumetric flux is obtained after the integration of x_3 in the bed-normal di-
 809 rection (Nelson & Fringer, 2018)

$$810 \quad \langle u'_3 \phi' \rangle_c - w \langle \phi \rangle_c - K \frac{\partial \langle \phi \rangle_c}{\partial x_3} = 0, \quad (A1)$$

811 where the three components on left-hand side (LHS) are the turbulent suspension flux,
 812 settling flux, and diffusive (molecular) sediment flux, respectively.

813 The time-averaged TKE budget equation is also used in this study, which reads (Reynolds
 814 & Hussain, 1972)

$$815 \quad \frac{\overline{D}\bar{k}}{Dt} + \nabla \cdot T' = \langle \mathcal{P}_c \rangle_c + \langle \mathcal{P}_w \rangle_c + \langle \mathcal{P}_b \rangle_c - \langle \varepsilon \rangle_c, \quad (A2)$$

816 where $\overline{D}/Dt = \partial/\partial t + \bar{u}_j \partial/\partial x_j$ and ε denotes the turbulence dissipation rate, which
 817 reads

$$818 \quad \varepsilon = \nu \frac{\partial u'_i}{\partial x_j} \frac{\partial u'_i}{\partial x_j}. \quad (A3)$$

819 The terms $\langle \mathcal{P}_c \rangle_c$, $\langle \mathcal{P}_w \rangle_c$ and $\langle \mathcal{P}_b \rangle_c$ on the RHS of equation (A2) represent the produc-
 820 tion (or destruction) of \bar{k} respectively from the mean current, waves and buoyancy, which

821 are written as

$$822 \quad \mathcal{P}_c = -\overline{u'_i u'_j} \frac{\partial \bar{u}_i}{\partial x_j}, \quad (A4)$$

$$823 \quad \mathcal{P}_w = -\langle u'_i u'_j \rangle_p \frac{\partial \langle u_i \rangle_w}{\partial x_j}, \quad (A5)$$

$$824 \quad \mathcal{P}_b = Rgn_i \phi' u'_i. \quad (A6)$$

825 Finally, the remaining terms in the plane- and time-averaged TKE budget equation (A2)
 826 are represented by $\nabla \cdot T'$.

827 For addressing the role of downslope gravity and wave direction in WSGF, the bal-
 828 ance equation of momentum flux in bed-normal direction is also needed. The derivation
 829 begins with the time- and plane-averaged momentum equation in the downslope direc-
 830 tion. This equation is derived from equation (2) after applying the time- and plane-averaging
 831 on both sides of it, which reads

$$832 \quad \nu \frac{\partial^2 \langle u_1 \rangle_c}{\partial x_3^2} - \frac{\partial \langle u'_1 u'_3 \rangle_c}{\partial x_3} \equiv \frac{\partial \langle \tau \rangle_c}{\partial x_3} = -Rgn_1 \langle \phi \rangle_c, \quad (A7)$$

833 where $\langle \tau \rangle_c$ is the total shear stress including the time- and plane-averaged viscous and
 834 Reynolds shear stresses. The above equation is consistent with the argument by Parsons
 835 et al. (2009) that the basic dynamics of WSGF are governed by the force balance in the
 836 cross-shelf direction between the downslope gravitational force (last term) and the ver-
 837 tical distribution of the total shear stress in the water column ($\partial \langle \tau \rangle_c / \partial x_3$). By vertically
 838 integrating equation (A7) from x_3 to the top of the computational domain and apply-
 839 ing the free-slip boundary condition on $x_3 = L_3$, we obtain the balance equation of mo-
 840 mentum flux in the bed-normal direction as

$$841 \quad \nu \frac{\partial \langle u_1 \rangle_c}{\partial x_3} = \langle u'_1 u'_3 \rangle_c + Rgn_1 \int_{x_3}^{L_3} \langle \phi \rangle_c dx', \quad (A8)$$

842 where x' is a dummy variable. A good agreement of the above momentum flux balance
 843 in present simulations has been found. The momentum flux balance equation (A8) com-
 844 bined with the sediment flux budget equation (A1) represents the dual role of wave-induced
 845 fluid turbulence in WSGF: providing energy to keep sediments in suspension which fur-
 846 ther forces the downslope current; contributing to the resistance (the component of Reynolds
 847 stress in the total shear stress) against the downslope driving force in the bottom bound-
 848 ary layer.

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 856 available via the repository maintained by GitHub: <https://github.com/yueliangyi/TURBID> (source code) and https://github.com/yueliangyi/TURBID/tree/master/spike/wave_supported_gravity_flow (case setup), respectively.

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Table 1. A list of simulations investigated in this study.

Case	θ (rad)	m_i^a	$L_1 \times L_2 \times L_3$ (Δ)	$N_1 \times N_2 \times N_3$	N_w^b (T_w)
0	0	$\begin{bmatrix} 1 & 0 & 0 \end{bmatrix}$	$60 \times 30 \times 60$	$256 \times 192 \times 193$	60
1	0.005	$\begin{bmatrix} 1 & 0 & 0 \end{bmatrix}$	$60 \times 60 \times 60$	$256 \times 256 \times 193$	60
2	0.005	$\begin{bmatrix} 0 & 1 & 0 \end{bmatrix}$	$60 \times 60 \times 60$	$256 \times 256 \times 193$	60

^a m_i represents the direction of waves while the bottom slope is fixed in the x_1 direction. ^b N_w is the total run-time of simulation in units of wave period.

Table 2. A summary of key averaged flow quantities for simulations investigated in this study.

Case	M^a (T_w)	$\langle \tau_b \rangle_c$ (Pa)	Ri^b	\overline{F}_g (g/cm ² s)	\overline{U}_{gf} (cm/s)
0	20	0.42	1.58×10^{-4}	–	–
1	20	0.43	1.63×10^{-4}	4.68×10^{-3}	0.93
2	20	0.43	1.64×10^{-4}	7.94×10^{-3}	1.66

^a M is the number of wave periods to the end of simulation where the

data analysis is taken. ^bThe bulk Richardson number is defined as

$\text{Ri} = Rg\Delta\overline{\Phi}/U_w^2$, which quantifies the sediment-induced density stratification (Ozdemir et al., 2010).

Table 3. Parameterization of WSGFs.

Case	U_w (cm/s)	U_g (cm/s)	B (cm 2 /s 2)	Ri_{cr}	β	C_D
1	56	0.59	30.73	9.80×10^{-3}	30.39	4.67×10^{-3}
2	56	0.96	30.82	9.83×10^{-3}	18.66	2.88×10^{-3}

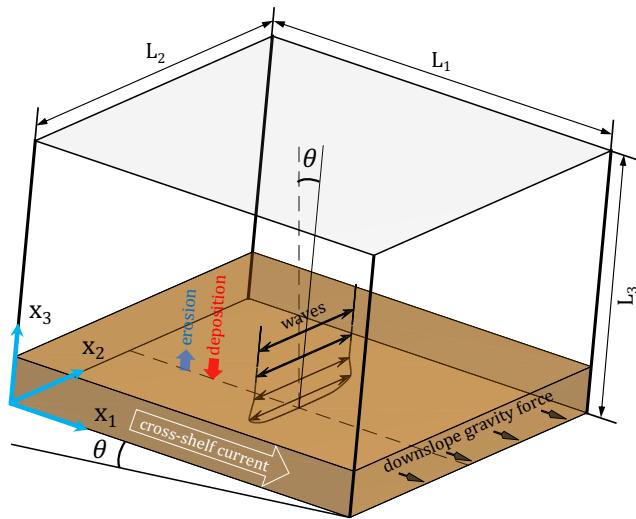


Figure 1. Sketch of computational domain and definition of the coordinate system.

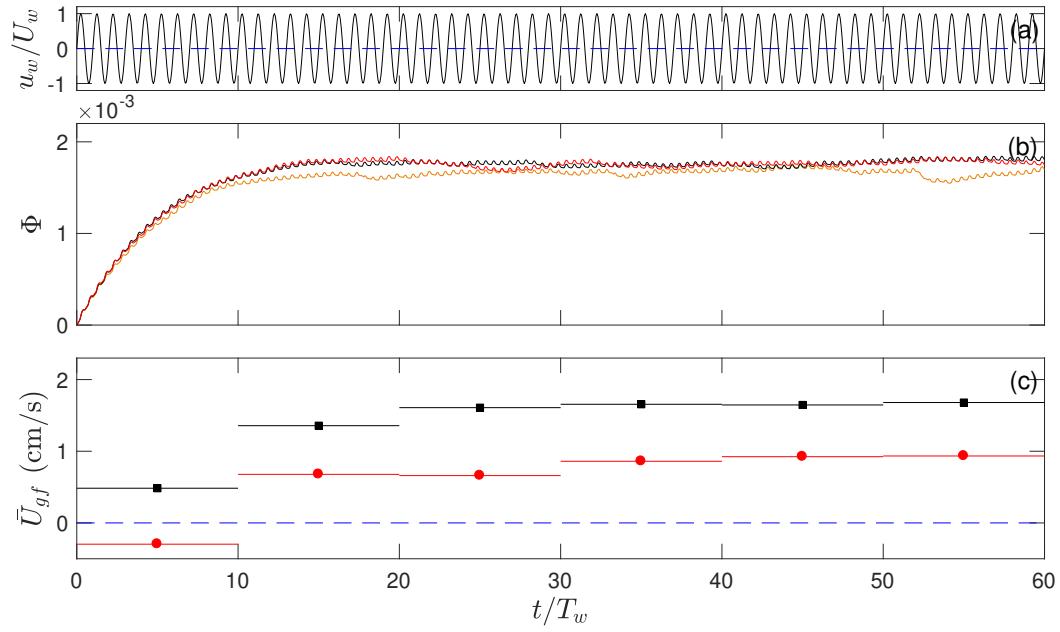


Figure 2. Time evolution of the (a) free-stream velocity; (b) domain-averaged volumetric concentration of sediment; (c) flux-based downslope current speed for Case 0 (brown line), Case 1 (red lines), and Case 2 (black lines). Blue dashed lines are plotted to indicate zero for reference.

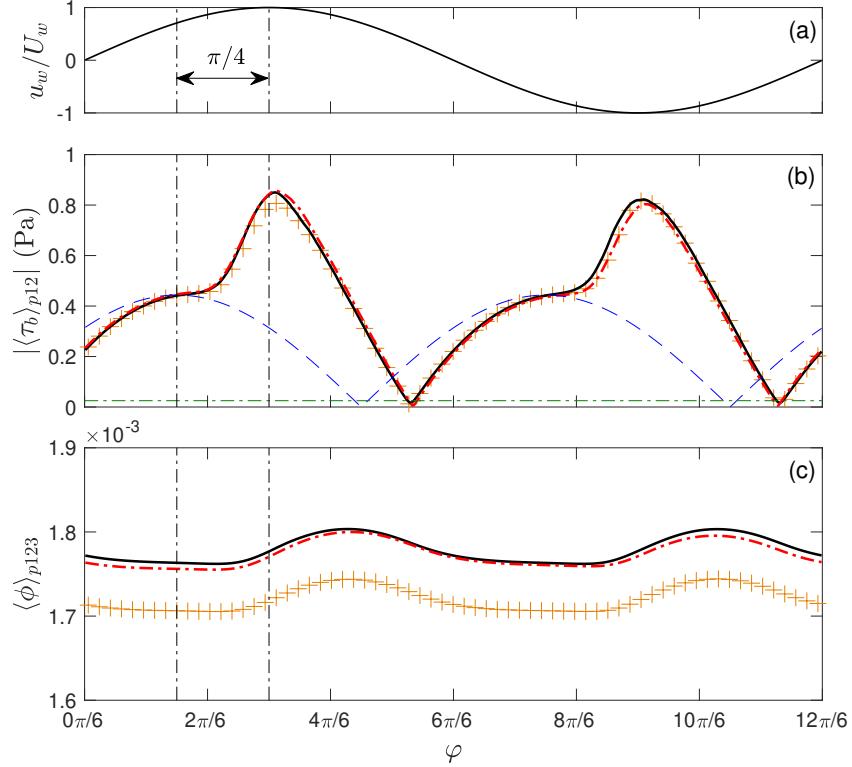


Figure 3. Time evolution of the (a) free-stream velocity; (b) amplitude of bottom shear stress; (c) domain-averaged volumetric concentration of sediment for Case 0 (brown crosses), Case 1 (red dash-dot lines), and Case 2 (black lines). The blue dashed line in subplot (b) is the corresponding laminar solution, while the green dash-dot line represents the critical bottom shear stress.

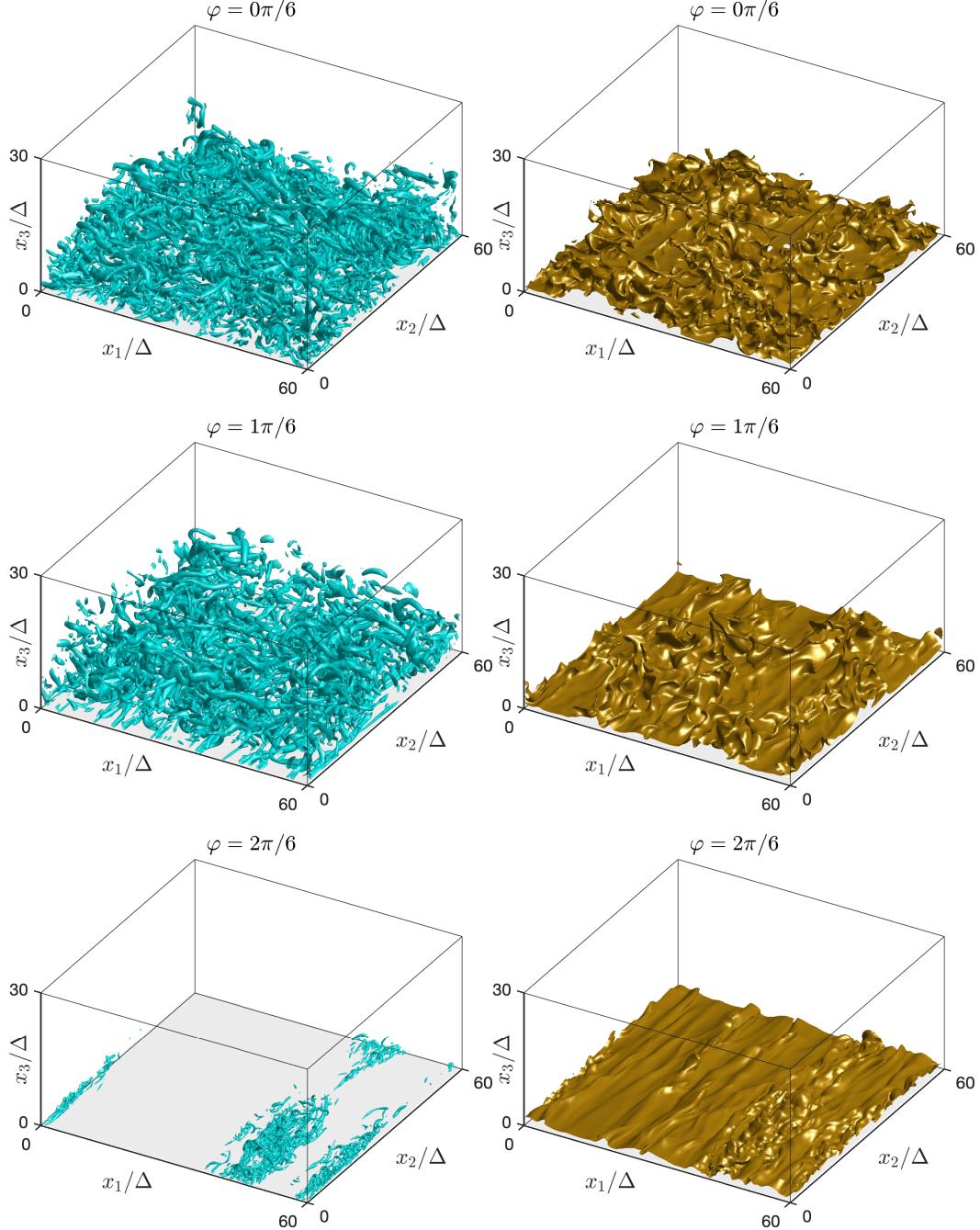


Figure 4. Coherent turbulent structures of flow (left) and corresponding isosurfaces of near-bed sediment concentration (right) of Case 2 at wave phases $\varphi = 0\pi/6$, $1\pi/6$ and $2\pi/6$. The coherent turbulent structures are visualized using the swirling strength (λ_{ci}), where the contour levels are set to be 10% of the corresponding maximum values with $\max(\lambda_{ci}) = 13.92$, 10.01 and 57.39 for the three phases, respectively. The contour levels of sediment concentration are chosen to be the averaged values at position $x_3 = 1.5$, which are 7.72×10^{-3} , 7.55×10^{-3} and 7.80×10^{-3} , respectively.

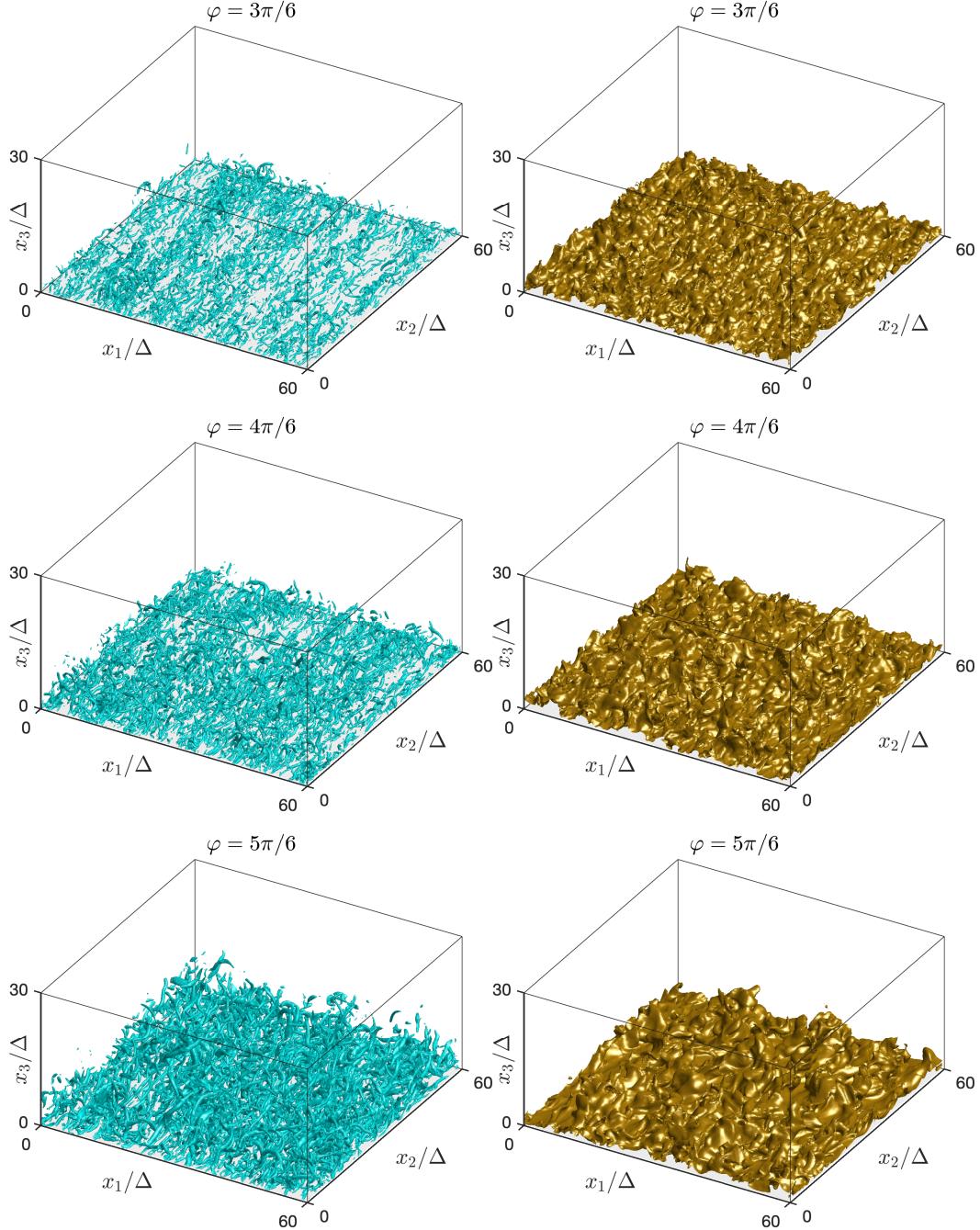


Figure 5. As Figure 4, but at wave phases $\varphi = 3\pi/6, 4\pi/6$ and $5\pi/6$. For the three phases, the maximum swirling strength are $\max(\lambda_{ci}) = 89.12, 57.98$ and 29.00 , while the contour levels of sediment concentration are $8.34 \times 10^{-3}, 8.37 \times 10^{-3}$ and 8.06×10^{-3} , respectively.

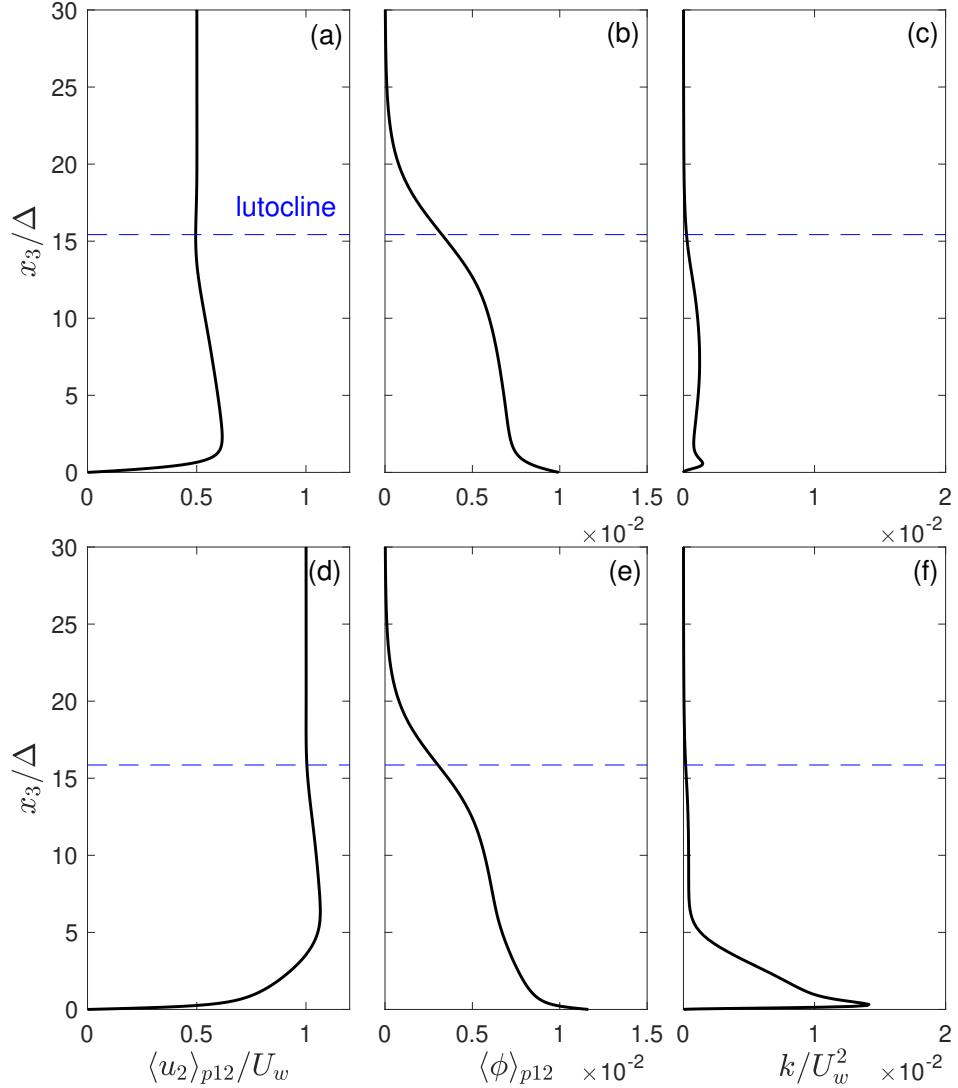


Figure 6. Plane- and phase-averaged (a,d) velocity in the direction of the waves; (b,e) volumetric concentration of sediment; (c,f) turbulence kinetic energy of flow for Case 2. Subplots in the first and second row represent results at wave phase $\varphi = \pi/6$ and $3\pi/6$, respectively.

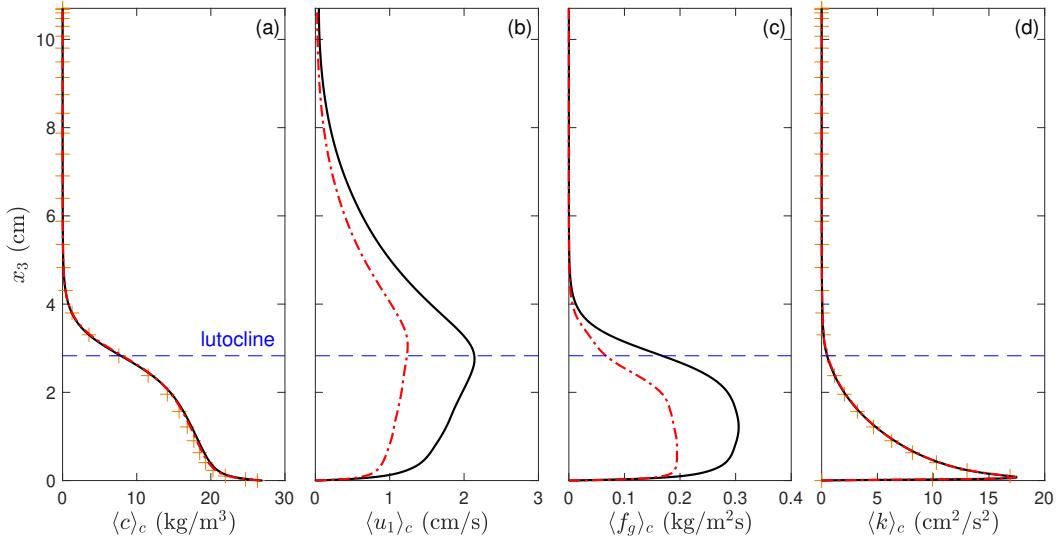


Figure 7. Vertical profiles of the time- and plane-averaged (a) mass concentration of sediment; (b) downslope current; (c) downslope mass flux of sediment; (d) turbulence kinetic energy for Case 0 (brown crosses), Case 1 (red dash-dot lines), and Case 2 (black lines).

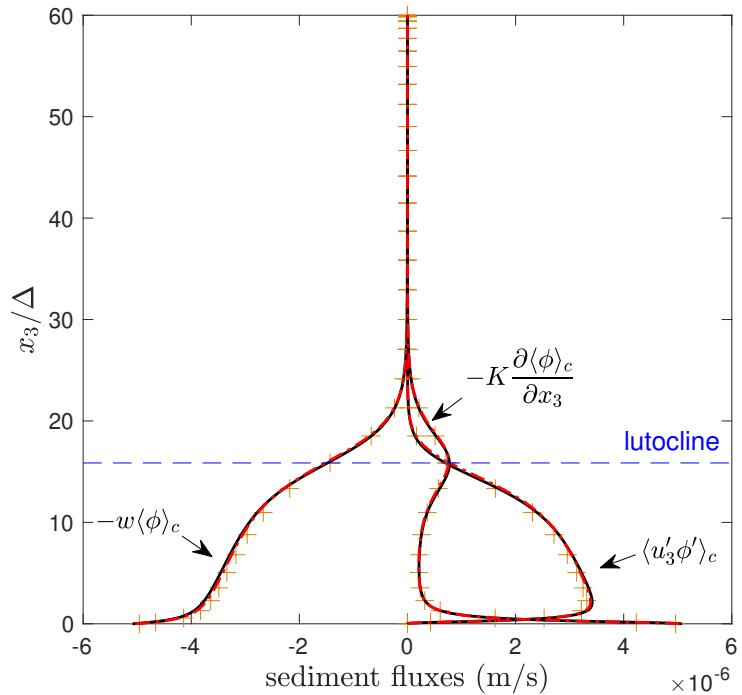


Figure 8. Flux budgets of sediment for Case 0 (brown crosses), Case 1 (red dash-dot lines), and Case 2 (black lines).

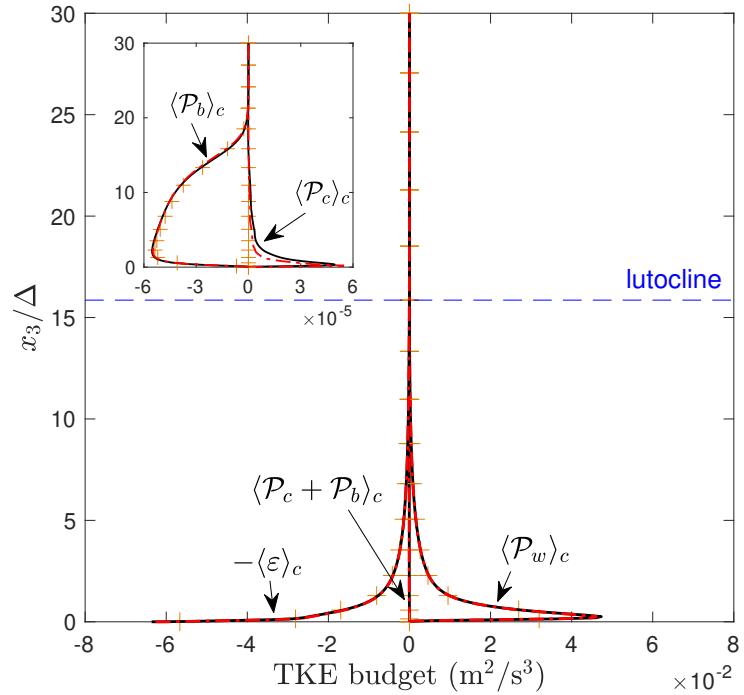


Figure 9. TKE budgets of the flow for Case 0 (brown crosses), Case 1 (red dash-dot lines), and Case 2 (black lines).

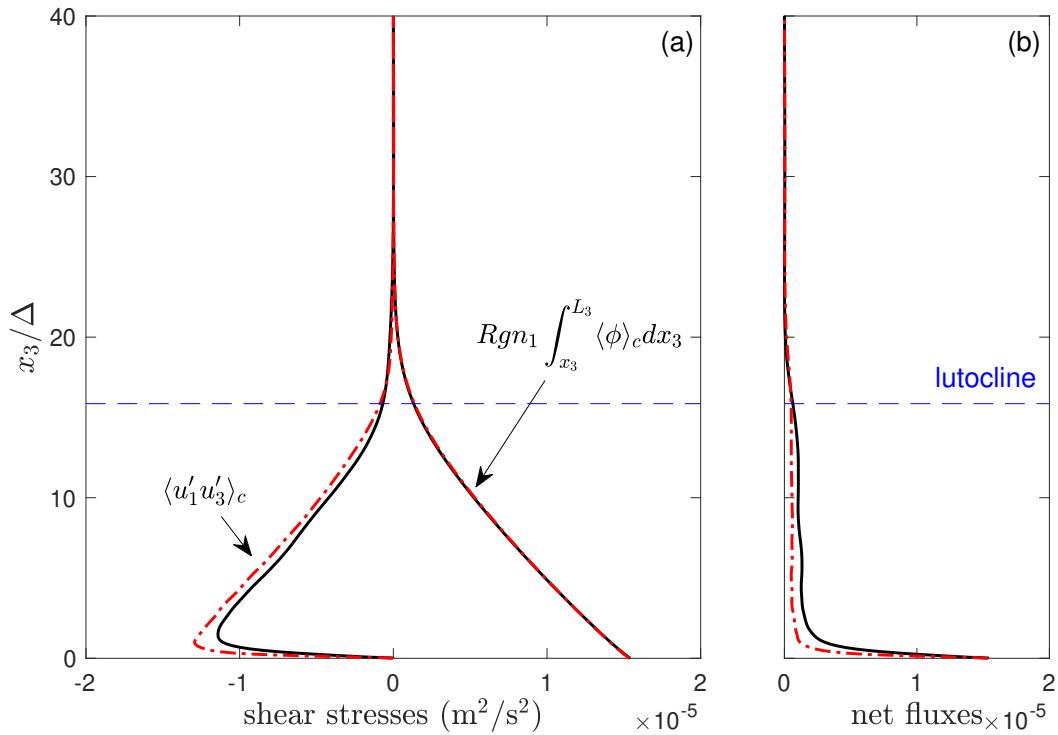


Figure 10. (a) Components in momentum flux balance in the bed-normal direction; (b) Net fluxes for Case 1 (red dash-dot lines) and Case 2 (black lines).

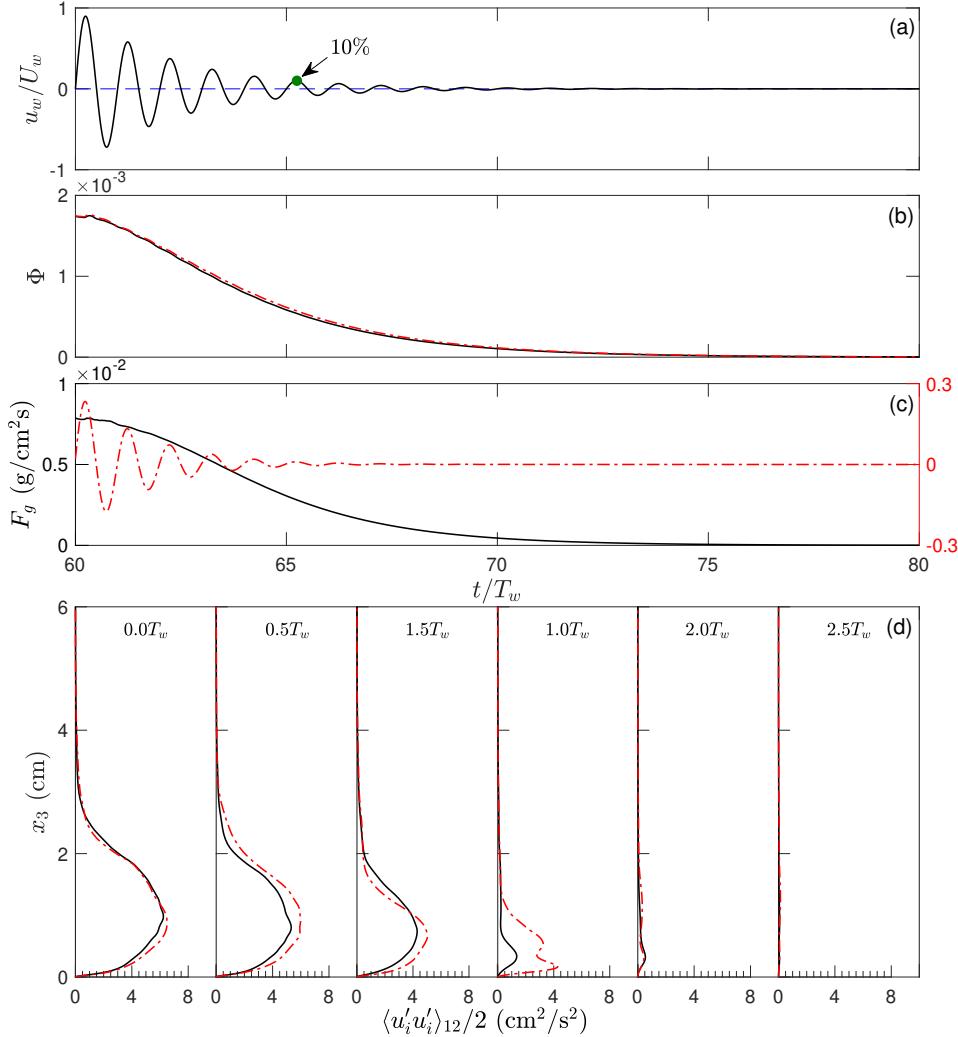


Figure 11. Time-history of the (a) damped free-stream velocity; (b) domain-averaged volumetric concentration of sediment; (c) domain-averaged mass transport rate of sediment in downslope direction. (d) Instantaneous profiles of turbulence kinetic energy at time $t - t_0 = [0.0 \ 0.5 \ 1.0 \ 1.5 \ 2.0 \ 2.5] T_w$ for Case 1 (red dash-dot lines) and Case 2 (black lines).