

Mechanics of Topologically Interlocked Material Systems under Point Load: Archimedean and Laves Tiling

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Abstract

Topologically interlocked material systems are two-dimensional assemblies of unit elements from which no element can be removed from the assembly without disassembly of the entire system. Consequently, such tile assemblies are able to carry transverse mechanical loads. Archimedean and Laves tilings are investigated as templates for the material system architecture. It is demonstrated under point loads that the architecture significantly affects the force-deflection response. Stiffness, load carrying capacity and toughness varied by a factor of at least three from the system with the poorest performance to the system with the best performance. Across all architectures stiffness, strength and toughness are found to be strongly and linearly correlated. Architecture characterizing parameters and their relationship to the mechanical behavior are investigated. It is shown that the measure of the smallest tile area in an assembly provides the best predictor of mechanical behavior. With small tiles present in the assembly the contact force network structure is well developed and the internal load path is channeled through these stiffest components of the assembly.

Keywords: Architectured Material Systems, Plates, Cross-property Relationships, Architecture-Property Relationships

¹ 1. Introduction

² Plates are ubiquitous two-dimensional structural units able to carry trans-
³ verse loads. Commonly, plates are monolithic [1], but plates-type structures
⁴ can also be assembled from topologically interlocked unit elements in the form
⁵ of convex polyhedra. Planar assemblies of convex polyhedra were considered
⁶ as early as in the 17th century [2]. A renewed interest in such structures

7 occurred recently in the civil engineering [3, 4, 5, 6, 7, 8, 9] and materials
8 engineering context [10, 11, 12, 13, 14, 15, 16].

9 In such assemblies individual building blocks (or tiles) are shaped and
10 arranged in the assembly such that no building block can be removed with-
11 out the disassembly of the overall system. When considering such systems
12 in the context of material design [17, 18, 19] they provide a unique method
13 to expand the material property space and for quasi-static loading has been
14 demonstrated to enable the transformation of a brittle response of a mono-
15 lithic plate made of brittle materials (such as ceramics, glasses or brittle
16 polymers) into a quasi-ductile response in the assembled plate-type struc-
17 ture [20, 21, 22]. Moreover, [23] demonstrated that for certain classes of
18 solid-architecture combination a simultaneous improvement of strength and
19 toughness of the assembled plate relative to the monolithic plate is possible.
20 Such favourable mechanical performance of the assembled plate structures
21 also were found to extend to impact loading by altering the relationship be-
22 tween impact velocity and residual velocity [24] and increased impact energy
23 absorption capacity [25, 26, 27]. In addition, assembled plate structures can
24 serve as the template for the implementation of adaptive structural configura-
25 tions [28, 29] to control system stiffness, strength and toughness.

26 In such prior work interlocked assemblies of building blocks have com-
27 monly been considered from the viewpoint of assemblies of all identical build-
28 ing blocks [30]. Such a viewpoint is limiting on what types of architectures
29 can be obtained. The material architecture can be expanded when the start-
30 ing point for the construction of the interlocked material system is an under-
31 lying grid instead of the particles [31, 32, 33, 34, 35, 36]. The construction
32 of topologically interlocked material systems emerging from underlying grid
33 systems is best placed into the context of the theory of tessellations [37] as
34 such an approach provides ordering principles for the architectures of con-
35 cern. The rules set forth in [38] are then applicable to realize the interlocking
36 building blocks related to a tessellation pattern.

37 The present study is connected to a background of prior work on the
38 mechanics of plate-type topologically interlocked assemblies. Prior work on
39 the mechanics of flat vaults [39, 40, 41, 42] has focused on the stability of
40 such systems under gravity loads while a second body of work has considered
41 applied displacement loads [43, 44, 45, 23, 46]. What has emerged is that
42 an understanding of the load-deformation response plate-type topologically
43 interlocked assemblies clearly cannot be conducted within the framework of
44 monolithic plates, and that the assembly architecture shall be an integral

45 part of the description of the respective mechanical response.

46 There has been an absence of systematic investigations into the mechanical
47 behavior of architected plate systems constructed on the basis of underlying grid systems (tessellations). This study seeks to fill this gap with
48 the ultimate objective to determine how the mechanical response of archi-
49 tected plates relates to the underlying tessellation patterns. All possible
50 Archimedean and Laves tilings are investigated. Cross-property relationships
51 between stiffness, strength and toughness [47] are determined as relationships
52 between the plate architecture and the plate mechanical response character-
53 istics.

55 2. Methods

56 2.1. Interlocking Assemblies

57 The midplane cross section of a topologically interlocked material (TIM)
58 system is a 2D tiling, and this tiling is considered as the basis for the cre-
59 ation of the TIM system [38]. TIM assemblies are considered as assemblies
60 of blocks (polyhedra) which have center sections conforming to the tilings.
61 Building blocks are constructed from the tiles of the tessellation by first pro-
62 jecting planes from each edge of the tile at alternating angles θ from the
63 normal. In the following the construction principle is reviewed. Without loss
64 in generality the principles are depicted for a square tiling. Code for the
65 generation of the respective geometries is available [48].

66 The magnitude of the edge projection angle θ is a fixed value, but its
67 direction will alternate between angling toward the tile center and away from
68 the tile center for each edge. The projection angle for all configurations in
69 this study was $\theta = 17^\circ$. Within an assembly, the blocks must be oriented such
70 that their edge projection angles are complimentary; if the edge of one tile
71 is angled toward the tile center, the abutting edge of the adjacent tile must
72 be angled away from its center. Once the projection angles are specified,
73 the vertices of the block can be determined. Each block formed from an n -
74 sided tile will have n vertices, and if the tile is a regular polygon, a uniform
75 antiprism block will be formed. Blocks constructed from tiles of different sizes
76 and shapes naturally have differing overall dimensions. In order to control
77 the aspect ratios of the TIM systems it is necessary to truncate the polyhedra
78 to possess a common top and bottom plane in an assembly. Two additional
79 planes must be defined parallel, at distance H_0 and equidistant from the
80 tiling plane. Each building block (i.e. the truncated polyhedra) formed from

81 an n -sided tile now possesses $2n$ vertices. Every set of planes projecting from
 82 two consecutive tile edges will yield two vertices, one by computing their
 83 intersections with the top plane, Fig. 1(a-b), and the other by computing their
 84 intersection with the bottom plane, Fig. 1(c-d). Computing the intersection
 85 of all sets of planes projecting from two consecutive edges and the top or
 86 bottom planes will locate all the vertices of the block, Fig. 1(e). Edges are
 87 then drawn between the vertices to construct the block, Fig. 1(f). In the
 88 interlocking assembly, Fig. 1(g), neighboring blocks impose constraints on
 89 each other such that assembly is load carrying.

90 *2.2. Tile Spaces*

91 The Archimedean and the Laves tilings are considered [37]. These tile sets
 92 are duals to each other. Archimedean tilings consist of regular polygons only
 93 and possess one type of vertex. Laves tilings are defined as having an equal
 94 angular spacing of all edges at any vertex [37]. There are 11 Archimedean
 95 and 11 Laves tilings. Their structure is described by the naming convention
 96 of [37]. Integer numbers with exponents separated by periods and contained
 97 within parenthesis describe the common vertex at all tile intersections such

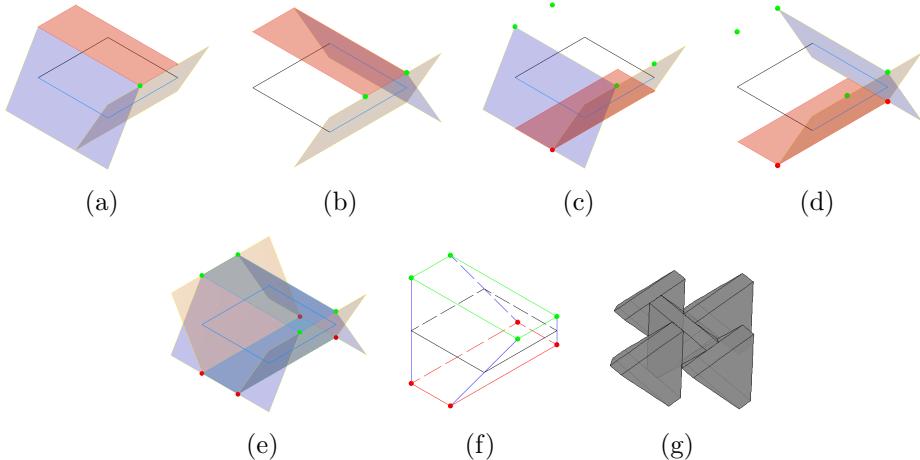


Figure 1: Truncated block construction from a square tile. (a) The intersection of two edge planes and the top plane defines the first top vertex. (b) The intersection of the next two edge planes and the top plane defines the second top vertex. (c) The intersection of two edge planes and the bottom plane defines the first bottom vertex. (d) The intersection of the next two edge planes and the bottom plane defines the next bottom vertex. (e) All planes and all vertices. (f) Wire frame of the resulting block. (g) Assembly of building blocks.

98 that each integer represents the number of sides of a tile that shares the
99 vertex, and the exponent is the number of that type of tile that shares the
100 vertex.

101 The Archimedean tilings are shown in Fig. 2(a). In a TIM system, the
102 sides of each block must alternate between sloping toward and away from the
103 normal to the plane of tessellation. Therefore, all tiles are required to possess
104 an even number of sides. This restriction eliminates the (3^6) , $(3^4.6)$, $(3^3.4^2)$,
105 $(3^2.4.3.4)$, $(3.4.6.4)$, $(3.6.3.6)$, and (3.12^2) tilings for consideration as a TIM
106 system. The remaining tilings from which TIM systems can be constructed
107 are (4^4) , (6^3) , $(4.6.12)$, and (4.8^2) .

108 The Laves tilings are shown in Fig. 2(b). Again, TIM systems can only be
109 constructed from a subset of the Laves tilings. The necessity for tiles with an
110 even number of sides when constructing a TIM system eliminates the $[3^4.6]$,
111 $[3^3.4^2]$, $[3^2.4.3.4]$, $[3.12^2]$, $[4.6.12]$, $[4.8^2]$, and $[6^3]$ tilings. The remaining tilings
112 are the $[3^6]$, $[3.6.3.6]$, $[3.4.6.4]$, and $[4^4]$ tilings. The $[4^4]$ and $[3^6]$ tilings are
113 regular tilings and are equivalent to the (4^4) and (6^3) regular Archimedean
114 tilings. Therefore, only the $[3.6.3.6]$ and $[3.4.6.4]$ tilings are added beyond
115 those from the Archimedean tilings.

116 In summary, the tilings suitable to TIM system construction are the (4^4)
117 (or $[4^4]$), $[3.6.3.6]$, $[3.4.6.4]$, (6^3) (or $[3^6]$), (4.8^2) , and $(4.6.12)$ tilings. The
118 Laves notation was chosen to denote the $[4^4]$ and $[3^6]$ tilings instead of the
119 Archimedean notation of (4^4) and (6^3) because these tilings are more simi-
120 lar to the other Laves tilings than to the other Archimedean tilings in this
121 study. The $[4^4]$, $[3.6.3.6]$, $[3.4.6.4]$, and $[3^6]$ tilings each consist of a single tile,
122 whereas the (4.8^2) tiling consists of two different tiles and the $(4.6.12)$ tiling
123 consists of three different tiles.

124 By their definition, tilings expand infinitely within a plane, yet here finite
125 size assemblies are considered. Boundaries in the form of a regular polygon
126 are defined for each tiling, such that the tiling was radially symmetric about
127 its center point within the boundary. Squares or hexagons meet this crite-
128 ria depending on the tiling but it is generally not possible to draw such a
129 boundary without crossing any of the tiles. In such cases, any tiles that were
130 intersected by the border became part of the border. Furthermore, there
131 are multiple possible center points for each tiling, such as centering the bor-
132 der around different vertices or around the centroid of different tiles. These
133 various boundaries are referred to as A, B, and C variants of a given tiling.

134 All tilings were made into plate equivalent structures of fixed thickness
135 value and aspect ratio. The thickness of all assemblies was set to $H_0=10.0$

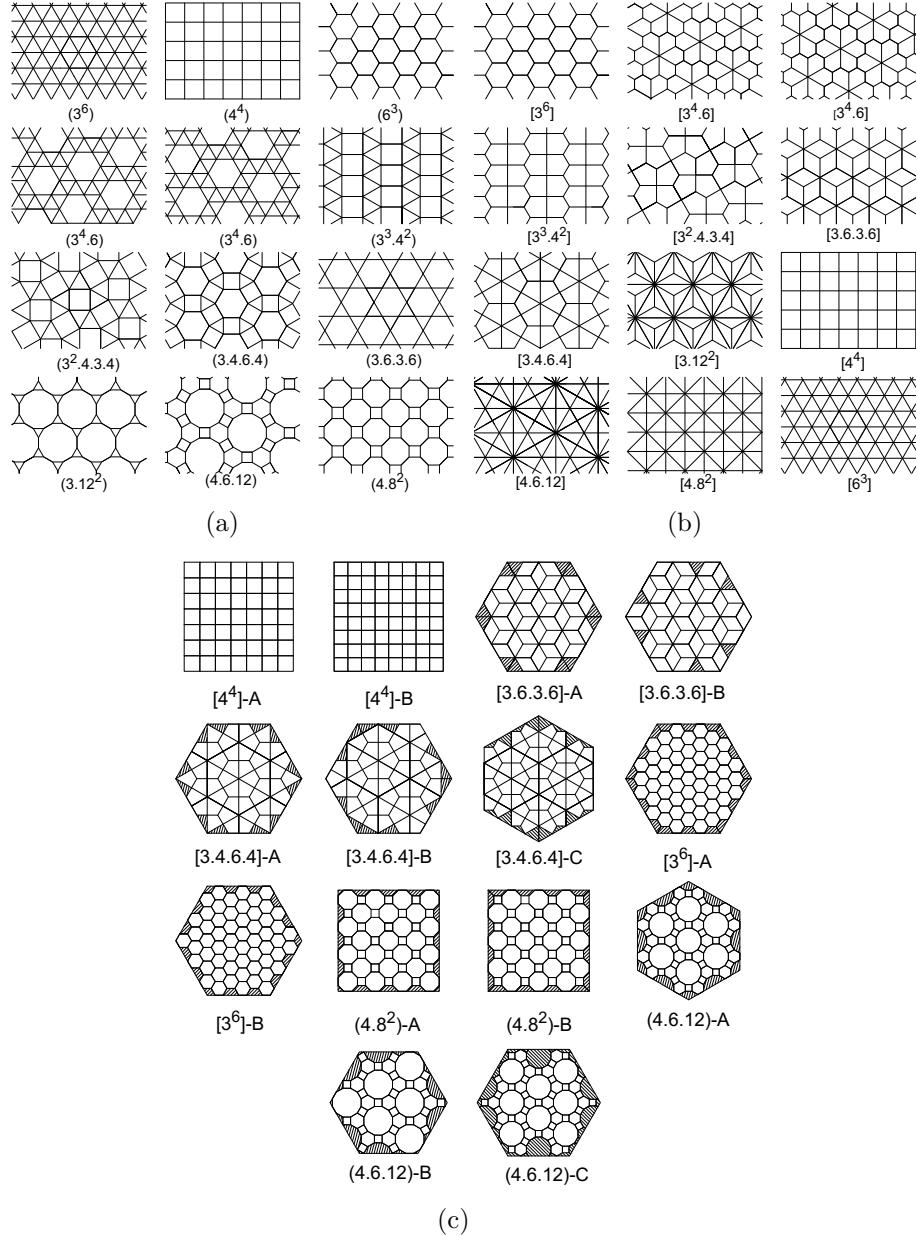


Figure 2: (a) The 11 distinct Archimedean tilings. The $(3^4.6)$ tiling occurs in two forms, both are shown here. (b) The 11 distinct Laves tilings. The $[3^4.6]$ tiling occurs in left-handed and right-handed forms, both are shown here. (c) The set of bounded tilings considered in this work.

Table 1: Number of tiles and edges lengths for the set of bounded tilings in this work.

| Tiling | Tiles | Edge 1 [mm] | Edge 2 [mm] |
|-----------------------|-------|-------------|-------------|
| [4 ⁴]-A | 49 | 29.7 | - |
| [4 ⁴]-B | 64 | 26.0 | - |
| [3.6.3.6]-A | 42 | 30.0 | - |
| [3.6.3.6]-B | 46 | 30.0 | - |
| [3.4.6.4]-A | 48 | 20.0 | 34.6 |
| [3.4.6.4]-B | 48 | 20.0 | 34.6 |
| [3.4.6.4]-C | 64 | 17.3 | 30.0 |
| [3 ⁶]-A | 55 | 15.0 | - |
| [3 ⁶]-B | 57 | 15.0 | - |
| (4.8 ²)-A | 49 | 12.2 | - |
| (4.8 ²)-B | 49 | 12.2 | - |
| (4.6.12)-A | 61 | 12.7 | - |
| (4.6.12)-B | 43 | 14.6 | - |
| (4.6.12)-C | 61 | 12.6 | - |

136 mm. Square and hexagonal shaped assemblies are identical in that L_0 is the
 137 radius of the circle inscribed into the square or hexagon, Fig. 2(c). Prior work
 138 [43] has shown that a minimum of 7 unit blocks per edge of the assembly
 139 is required to create TIM systems suitable for investigation. The value of
 140 the in-plane dimension L_0 was derived for the geometric constraints imposed
 141 by the (4.6.12)-C assembly. This assembly, by nature of the combination of
 142 large and small building blocks imposes an upper limit on L_0 . Geometric
 143 constraints for the (4.6.12)-C assembly with 61 blocks lead to an assembly
 144 having the ratio $L_0/H_0 = 10.39$. This value of L_0/H_0 is then imposed on
 145 all other assemblies. In addition, the condition of 10% truncation of the
 146 smallest building block type in an assembly was desired to maintain flat top
 147 and bottom planes.

148 Ideally, all tilings would be constructed to have the same number of block
 149 in each assembly. However, the tiling structure imposes geometric constraints
 150 that such a condition cannot be met within a fixed L_0/H_0 value and the
 151 resulting bounded tilings range from 42 tiles up to 64 tiles, Table 1 and
 152 Fig. 2(c). Table 1 lists all tile edge lengths values. The [3.4.6.4] tiling is the
 153 only tiling considered in this study possessing than one edge length value.

154 TIM systems require a bounding frame (fence) for constraint. The bound-
 155 ing frames were constructed by expanding each tiling beyond the boundaries

156 drawn in Fig. 2(c) such that there exists a tile adjacent to every side of the
157 outer tiles in the bounded set. Blocks were generated on these additional
158 tiles such that the blocks formed from the bounded tiling were completely
159 surrounded by this additional set of blocks. The blocks in the outermost set
160 were fused into a single part to serve as a frame for the assembly. The outer
161 profile of this conglomerate frame was cut into either a square or hexagon
162 shape as appropriate.

163 The geometry of the single-tile systems is such that they can be flipped
164 over and rotated to exactly overlay their original position. However, the
165 multi-tile systems do not typically share this property. The TIM system
166 configurations used in this study are named after the bounded tiling from
167 which they were created, and if the response of the assembly is direction
168 dependent, the load direction will be indicated. For example, the [4⁴]-A
169 assembly is not direction dependent, but the [4.8²]-A assembly is, therefore
170 it will be denoted as two separate configurations [4.8²]-A(-) and [4.8²]-A(+).
171 The complete set of TIM system configurations in this study is shown in
172 Appendix A.

173 2.3. Analysis

174 Finite element models are created for the analysis of the transverse force-
175 deflection response. The bounding frame is considered as rigid and is fixed
176 in space. Displacement boundary conditions are imposed via a rigid indenter
177 pin interacting via contact located centrally to the assembly. A monotonically
178 increasing displacement is applied to the indenter. Individual building
179 blocks are linear elastic and interact with each other by contact and friction.
180 Details of the analysis approach are provided in Appendix B. Calculations
181 provide the force (F) -deflection (u) response of assemblies computed as the
182 respective data on the indenter. The F - u response is depicted as both raw
183 and filtered data. System characteristic points are marked on the F - u plots
184 and these were extracted from each simulated configuration:

- 185 1. Stiffness as the secant to 80% of the maximum force,
- 186 2. Strength as the maximum force recorded,
- 187 3. Displacement u_{50} at the point the force drops to 50% of its maximum
188 value,
- 189 4. Toughness as the integral of F - u up u_{50} , and
- 190 5. Displacement u_{slip} at the point the magnitude of the frictional dissipa-
191 tion becomes greater than the strain energy (ALLFD > ALLSE).

192 In all computations the mechanical response is dominated by the strain
 193 energy and friction dissipation such that the external work **ALLWK** is the sum
 194 of strain energy **ALLSE** and friction dissipation **ALLFD**. All other contributions
 195 (penalty work in the contact **ALLPW**, viscous dissipation **ALLVD**, artificial ener-
 196 gies **ALLAE** and kinetic energy **ALLKE**) are negligible, at least up to conditions
 197 where slip starts to dominate and **ALLSE** < **ALLFD**.

198 **3. Results**

199 Results for computations for (*i*) the [3.4.6.4]-B system with a single tile
 200 type, Fig. 3(a), (*ii*) the [4.8²]-A(+) system with two different tiles, Fig. 4(a) ,
 201 and (*iii*) the [4.6.12]-A(+) system composed of three different tiles, Fig. 5(a),
 202 are depicted as representative for the computational study.

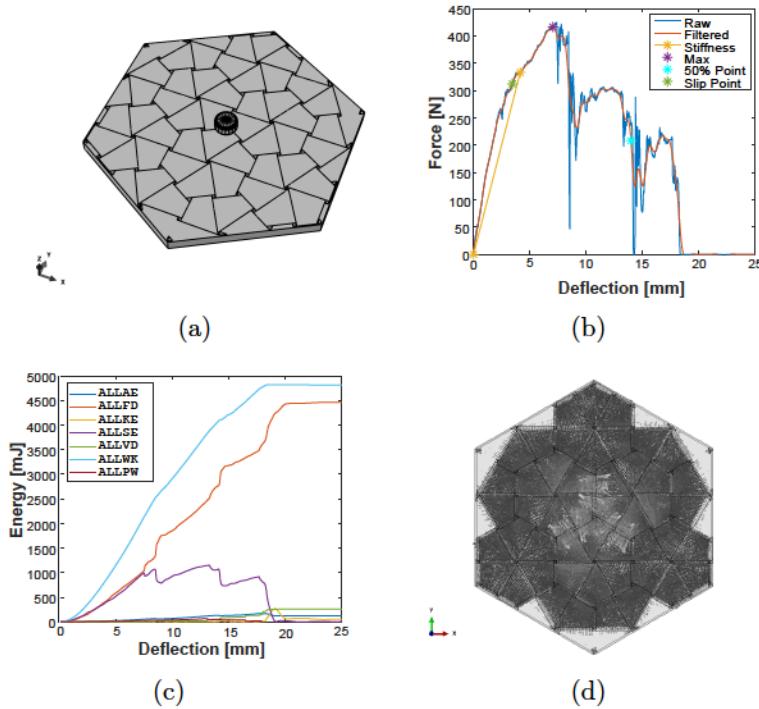


Figure 3: (a) The [3.4.6.4]-B TIM system constructed with one tile type, (b) System energies, (c) Force-deflection response, (d) Vector plot of compressive principal stresses σ_{p3} at the maximum load $\sigma_{p3} = [-28, +1]$ MPa.

203 The *F-u* curves (Figs 3(b), 4(b), 5(b)) overall possess the skewed parabola
 204 shape with a gradual load decrease past the maximum load, similar to what

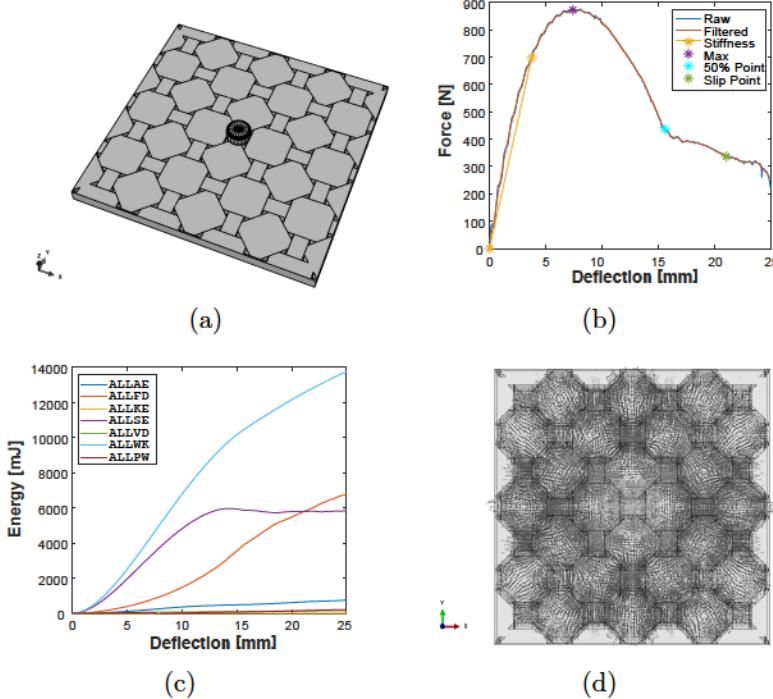


Figure 4: (a) The $[4.8^2]$ -B TIM system constructed with two tile types, (b) System energies (c) Force-deflection response, Vector plot of compressive principal stresses σ_{p3} at the maximum load with $\sigma_{p3} = [-52, +2]$ MPa.

205 has been documented in other investigations on TIM systems [20, 21, 22, 23].
 206 Stiffness, strength, toughness, the rate of force drop post the load maximum,
 207 and the slip onset vary distinctly between assembly architectures. Initially,
 208 the $F-u$ curves are smooth and deformation is by tilting of unit blocks and
 209 their elastic deformation. As deformation progresses, local slip events become
 210 apparent in the $F-u$ curve as intermittent load drops.

211 The three examples depicted represent conditions where the onset of slip
 212 dominance is significantly different. The contribution of slip to the defor-
 213 mation response can be assessed from the evolution of the systems energies
 214 during loading, Figs 3(c), 4(c), 5(c). For the $[3.4.6.4]$ -B case, slip is a strongly
 215 dominant factor. Friction dissipation is of equal magnitude as the strain en-
 216 ergy already during early stages of loading, and slip becomes dominant past
 217 the maximum load at $u = 7.58$ mm. For the $[4.8^2]$ -B configuration and the
 218 $[4.6.12]$ -A(+) case, the strain energy is much larger than the frictional dis-

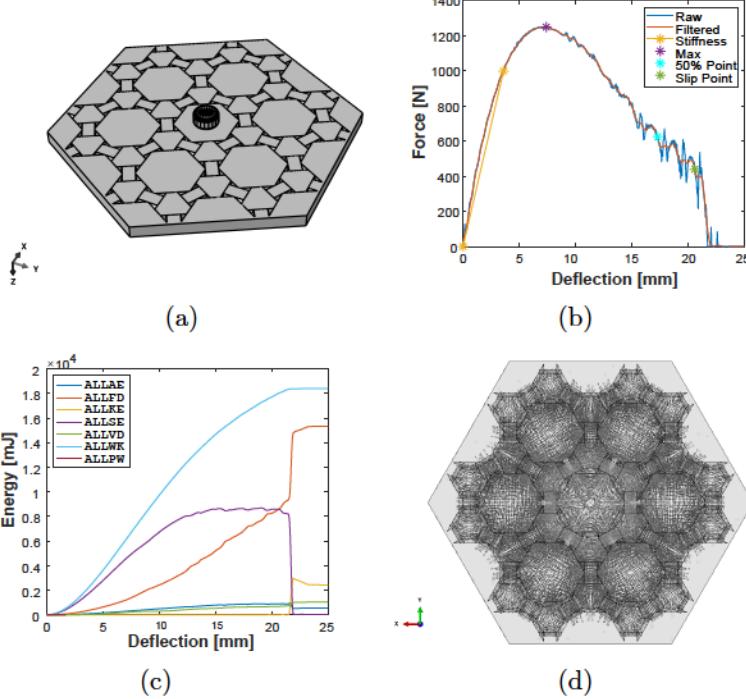


Figure 5: (a) The [4.6.12]-A(+) TIM system constructed with three tile types, (b) System energies, (c) Force-deflection response, (d) Vector plot of compressive principal stresses σ_{p3} at the maximum load with $\sigma_{p3} = [-120, +1]$ MPa.

219 sipation over much of the load history. The slip onset condition is delayed
 220 to $u_{slip}=21.05$ mm for the [4.8²]-B case and to $u_{slip}=20.6$ mm [4.6.12]-A(+)
 221 case, far into the deformation histories. Slip alone is not the sole determining
 222 factor for the strength of a system. While the [3.4.6.4]-B with the largest
 223 slip contribution also possesses the lowest strength $F_{max}=415.9$ N, the two
 224 other systems possess distinctly different strength despite similarly delayed
 225 slip: F_{max} for [4.8²]-B is 872.7 N and for [4.6.12]-A(+) it is 1249.0 N. Past
 226 the maximum load, failure is gradual at least until the latest stages of the
 227 deformation history. For the [3.4.6.4]-B assembly, strong local intermittent
 228 load drops are associate with slip events and load carrying capacity is lost
 229 early, ($u_{50}=14.0$ mm). For the other two assemblies slip events are less pro-
 230 nounced in the $F-u$ data, and u_{50} values are significantly larger: $u_{50}=15.6$
 231 mm for [4.8²]-B and 17.3 mm for [4.6.12]-A(+). As a consequence, tough-
 232 ness values are also significantly different. The toughness is the least for

233 [3.4.6.4]-B, followed by [4.8²]-B and [4.6.12]-A(+).

234 In TIM systems, load transfer is dominated by compressive loads in building
235 blocks balanced by tensile loads in the bounding frame. The computed
236 load transfer patterns in the assemblies are depicted as vector plots of the
237 compressive principal stress σ_{p3} at the state of maximum load. In the as-
238 semby [3.4.6.4]-B the distribution of σ_{p3} is found to be rather homogeneous
239 throughout and the entire assembly perimeter transfers load to the bounding
240 frame, Fig. 3(d). For the assembly [4.8²]-B it is found that σ_{p3} is less in the
241 larger tiles than it is in the smaller ones and a distinct load transfer pattern
242 is seen, Fig. 4(d). Now loads are transferred to the frame only along a subset
243 of faces to the bounding frame but both types of tiles contribute. The find-
244 ing of load transfer being dominant via the smallest building blocks is also
245 present in the results for the [4.6.12]-A(+) assembly, Fig. 5(d). In this case
246 load transfer to the frame is found to occur predominantly via the faces of
247 the smallest building blocks.

248 Subsequently, the characteristics of all computed configurations are con-
249 sidered in the form of cross-property relationships. Strength and stiffness
250 were linearly correlated to a high degree, Fig. 6(a). Stiffness and toughness,
251 Fig. 6(b), ($R^2=0.65$) as well as strength and toughness, Fig. 6(c), ($R^2=0.80$)
252 are also linearly correlated, but at a somewhat smaller R^2 value. From the
253 results Figs 3, 4 and 5 it could be inferred that the prevalence of slip would
254 be a good predictor of TIM properties. However, this was found to be only
255 partially the case. Strength is related to u_{slip} but the correlation is weak,
256 Fig. 6(d) ($R^2=0.64$). The relationship between stiffness and u_{slip} is even
257 weaker at $R^2 = 0.43$.

258 4. Discussion

259 The TIM plate systems introduced in this study exhibit attractive prop-
260 erties in terms of their failure. A gradual decrease in load is realized even if
261 the material used to make the building blocks would be considered as brittle
262 itself. Such a response is found across all system architectures considered.

263 The overall transverse force-deflection response of TIM systems has been
264 explained by the formation of multiple force chains in the granular-like as-
265 semby. Such force chains reach from the top plate face at which the load is
266 applied to the opposing bottom plate face. As the plate deflection increases
267 so do the angles between the force chains and the plate reference plane. Such

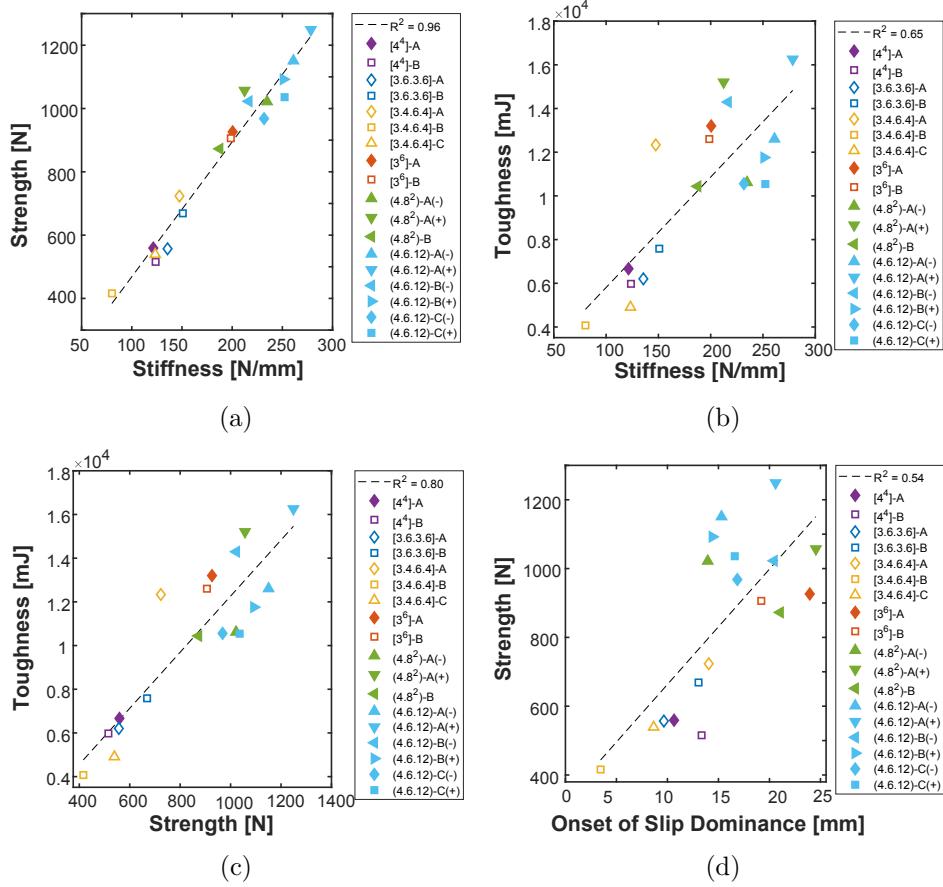


Figure 6: Cross property relationships: (a) Strength and stiffness, (b) Toughness and stiffness (c) Toughness and strength (d) Stiffness and onset of slip dominance.

268 a process is similar to what happens in a Mises-truss. In [45] a comprehensive
 269 model for this approach is demonstrated. Thus, the Mises-truss model
 270 is used to describe the force-deflection response of TIM plates, rather than
 271 a monolithic plate theory. Within the Mises-truss model, the observation
 272 of a linear dependence of strength on stiffness is consistent. The finding of
 273 rather linear relationships between stiffness and toughness and also between
 274 strength and toughness again relates well to the Mises-truss model, as does
 275 the observation that the displacement to final failure is rather constant and
 276 in the range of 2.0 - 2.5 times the assembly thickness. The variations in
 277 strengths (from about 400 to 1250 N) result in a vertical stretching of the
 278 skewed parabola $F-u$ response in which the stiffness and strength vary while

279 the deflection to failure does not.

280 It is of interest to relate the system mechanical characteristics to the
281 architectural aspects in order to find predictors of system performance. As
282 strength is well correlated with both stiffness and toughness, a predictor
283 for strength is also capable of predicting stiffness and toughness. To start,
284 strength was correlated against the number of tiles in an assembly, Table 1.
285 It was desired that there not be a correlation between these two parameters.
286 With a coefficient of determination $R^2 = 0.02$, this goal was met. Next,
287 strength was correlated against segmentation in the assembly.

288 One measure of the degree of segmentation is the total contact area be-
289 tween all segmented bodies in the assembly. The total contact area between
290 all segmented bodies in the assembly is computed in the assembly's initial po-
291 sition before any displacement had occurred. Strength and total contact area
292 between all segmented bodies in the assembly are well correlated, Fig. 7(a)
293 ($R^2 = 0.77$). Smaller values of total contact area lead to higher strength.
294 This suggests that the less segmented a structure is, the greater its strength
295 will be. This argument would intuitively agree with the fact that a monolithic
296 plate is generally stronger than its segmented counterparts.

297 A second measure of the degree of segmentation is the number of contact
298 interfaces in the assembly, defined as a state of contact between any two
299 bodies. Strength and the number of contact interfaces are less significantly
300 related, Fig. 7(b) ($R^2 = 0.61$), but an increase in the number of contact
301 interfaces is correlated to an increase in strength suggesting that the more
302 segmented a structure is, the greater its strength will be.

303 The correlations between strength and the two measures of segmentation
304 are in obvious disagreement. Clearly the degree of segmentation alone is
305 insufficient in predicting the properties of the TIM systems under considera-
306 tion here. The present data suggests that TIM behavior must be dependent
307 on how the system is segmented rather than how much it is segmented.

308 The assemblies having a larger number of contact interfaces did so by
309 having building blocks with a greater number of sides. It is possible to
310 increase the number of contact interfaces by increasing the number of building
311 blocks, but the TIM systems in this study all had approximately the same
312 number of building blocks. Therefore, the increase in strength seen with
313 the increase in contact interfaces might be attributed to the presence of
314 larger building blocks, rather than to the increase in contact interfaces. This
315 might describe the gap in strength values that is seen between the weakest
316 configurations (all based on tessellations with a single four-sided tile) and

317 the other configurations which also include larger tiles with a larger number
318 of sides, Fig. 7(b).

319 To further investigate the dependence of the mechanical behavior on sys-
320 tem architecture, strength is correlated against the area of the largest tile
321 in the tessellation from which each TIM system was constructed, Fig. 7(c).
322 Strength is found as positively correlated with the largest tile area in the
323 assembly ($R^2 = 0.52$). This finding does support the previous conjecture
324 that TIM systems with larger blocks would be stronger, but with a coeffi-
325 cient of determination $R^2 = 0.52$ it is not a strong correlation. Given that
326 the bounded tilings in this study all have approximately the same number
327 of tiles and about the same total area, if there are larger tiles in a tiling, it
328 must also possess some smaller tiles. Thus, strength is correlated against the
329 area of the smallest tile in the tessellation from which each TIM system was
330 constructed, Fig. 7(d). This relationship possesses a coefficient of deter-
331 mination $R^2 = 0.73$ and suggests that the smallest tile size is a better predictor
332 of strength than the largest tile size.

333 The findings on architecture-property relationships suggest that the strongest
334 TIM systems are the ones with the least total contact area, the greatest
335 number of contact interfaces, and the smallest tiles. This combination of
336 characteristics leads to the conclusion that TIM system configurations hav-
337 ing architectures that constrict load transfer into well defined force chains
338 possess the greatest strength. These increasing degrees in the concentration
339 of force chains are well represented in the results of the computations for the
340 TIM configurations based on the [3.4.6.4]-B, [4.8²]-B, and [4.6.12]-A tilings,
341 Figs 3(d), 4(d), 5(d). A more distinctly developed force chain network is
342 thereby not related only to the presence of smaller tiles in the assembly but
343 also to the geometry of the tessellation as the force chain network structure
344 develops within a specific assembly.

345 The present results were confined by two constraints: assemblies are pla-
346 nar and the interlocking geometry is based on planar tile faces. Neither
347 constraint is seen as a limitation in the application of present results. The
348 geometric arguments on tessellations and the resulting assemblies overall can
349 certainly be extended to curved systems made of topologically interlocked
350 building blocks which have recently been demonstrated in the context of
351 digital design and manufacturing approaches [34, 49, 50]. As it has been
352 demonstrated that osteomorphic shaped interlocking [51, 52] and multiscale
353 interlocking [53] provide similar or improved mechanical response as planar
354 type interlocking, it can be argued that the present results will be applicable

355 to such systems as well.

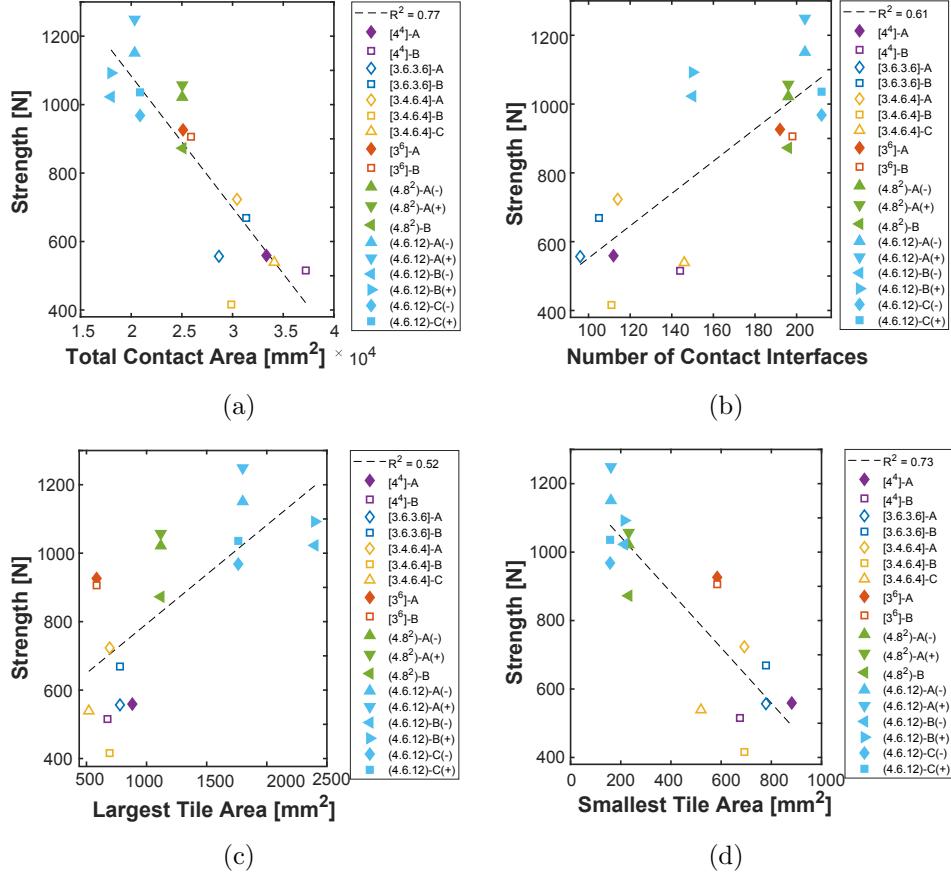


Figure 7: (a) Strength vs total contact area between segmented bodies; (b) Strength vs number of contact interfaces between segmented bodies; (c) Strength vs area of the largest tile in the base tiling; (d) Strength vs area of the smallest tile in the base tiling.

356 5. Conclusion

357 TIM systems were constructed for 18 configurations based on six unique
 358 tilings and their response under transversely applied displacement load is
 359 investigated. It was found that the load responses of all configurations were
 360 generally consistent with the typical skewed parabola that has been recorded
 361 in other TIM systems. The attractive positive correlations of toughness-
 362 stiffness and toughness-strength were realized for all configurations. There

363 exists significant variance in the performance of the TIM systems in this
364 study. It was generally observed that the triple-tile (4.6.12) configurations
365 were the strongest, followed but the double-tile (4.8²) configurations, the
366 single hexagon tile [3⁶] configurations, and then all the single four-sided tile
367 configurations. The stiffest, strongest, and toughest configurations tended
368 to have the least total contact area between segmented bodies, the greatest
369 number of contact interfaces, and the smallest tiles. It is postulated that this
370 combination of features leads to more confined force chains of the internal
371 load transfer. The findings of this study allow for an expansion of the mate-
372 rial space. When considering a segmented material system, a greater range
373 of ductility is available as compared to homogeneous materials. The tessel-
374 lation pattern can be chosen to achieve the desired ductility. These methods
375 can be used to design advantageous material systems that are ductile as a
376 system while maintaining high strength within the individual components.

377
378 **Acknowledgment:** This work supported by the National Science Founda-
379 tion under Grant No. 1662177.

380 **Appendix A. TIM Assemblies**

381 Figures A.8 to Fig. A.13 depict the TIM systems under consideration.

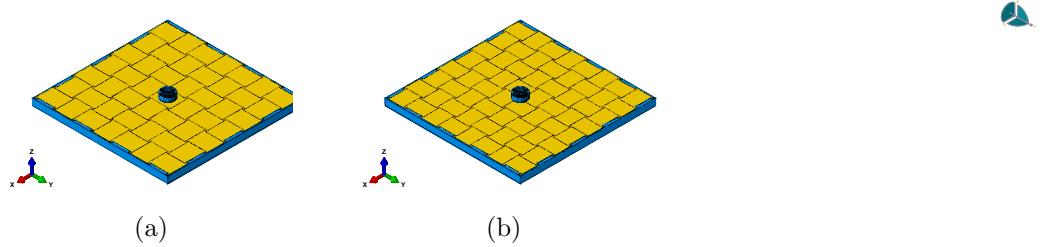


Figure A.8: Assembly configurations: (a) $[4^4]$ -A, (b) $[4^4]$ -B.

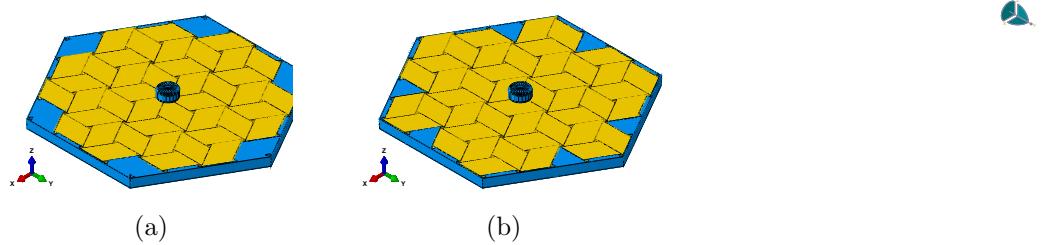


Figure A.9: Assembly configurations: (a) $[3.6.3.6]$ -A, (b) $[3.6.3.6]$ -B.

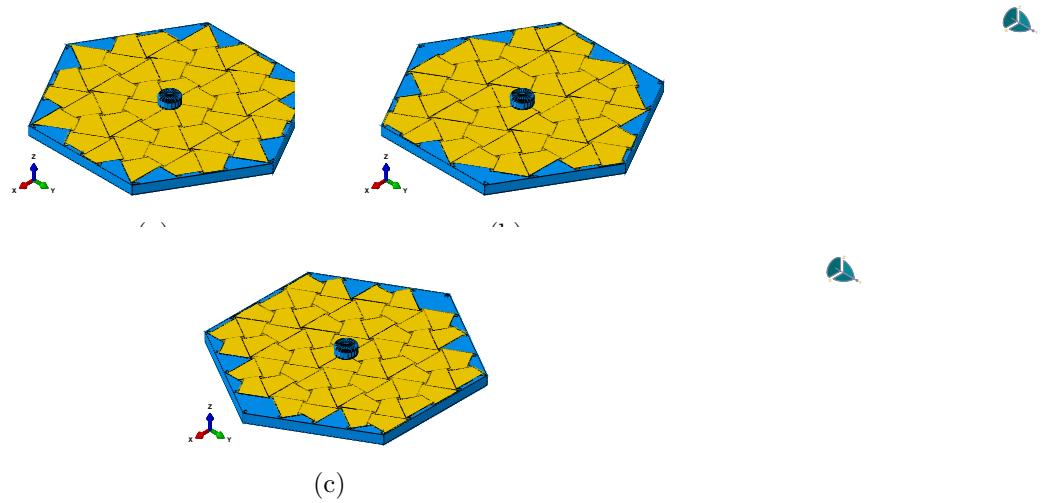


Figure A.10: Assembly configurations: (a) [3.4.6.4]-A, (b) [3.4.6.4]-B, (c) [3.4.6.4]-C.

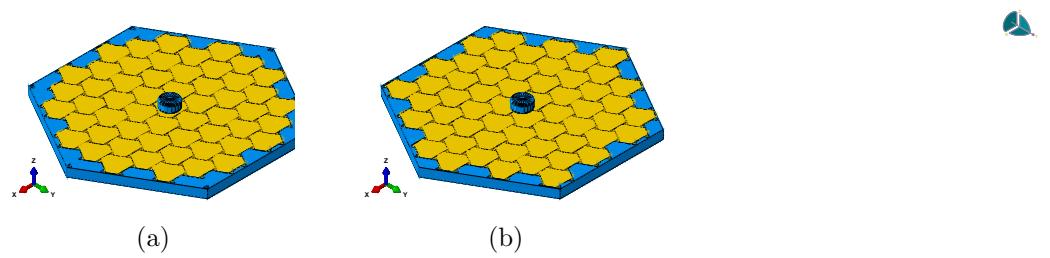


Figure A.11: Assembly configurations: (a) $[3^6]$ -A, (b) $[3^6]$ -B.

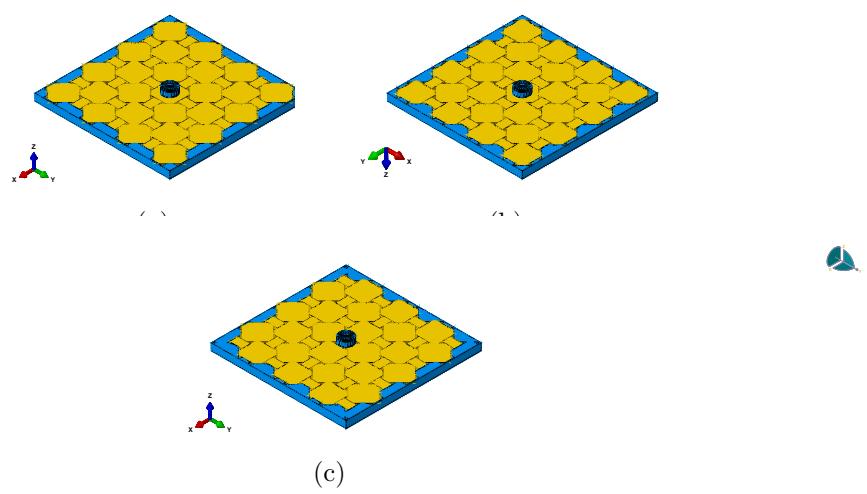


Figure A.12: Assembly configurations: (a) (4.8^2) -A(-), (b) (4.8^2) -A(+), (c) (4.8^2) -B.

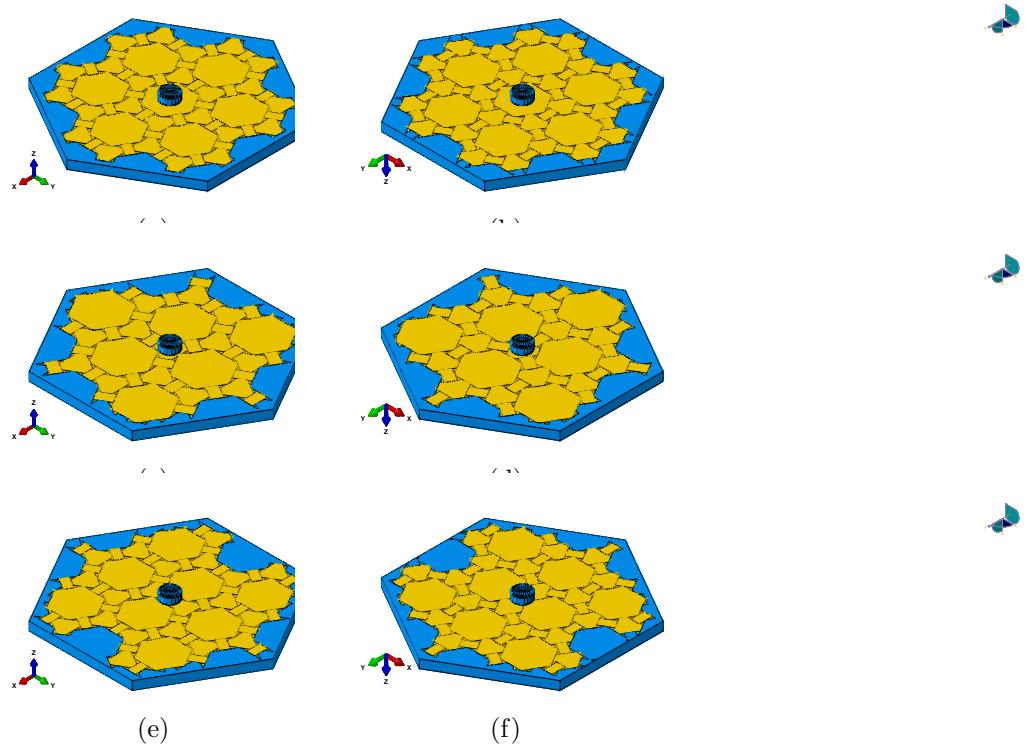


Figure A.13: Assembly configurations: (a) $(4.6.12)$ -A(-), (b) $(4.6.12)$ -A(+), (c) $(4.6.12)$ -B(-), (d) $(4.6.12)$ -B(+), (e) $(4.6.12)$ -C(-), (f) $(4.6.12)$ -C(+).

382 **Appendix B. Analysis Approach**

383 All model configurations are analyzed within the context of the finite
384 element analysis. Finite element models were computed with an explicit
385 solver (ABAQUS) to obtain the load - displacement response of all TIM
386 configurations under quasi-static transverse loading from point load under
387 displacement control conditions. The computed reaction forces were filtered
388 using a second order Butterworth filter with a cutoff frequency of 50 Hz.

389 The frame was made to be undeformable and fixed in space. The elastic
390 modulus of the unit elements was assigned to be $E = 1.83$ GPa, the Poisson
391 ratio $\nu = 0.35$. These properties are motivated by a 3D printing manu-
392 facturing approach for the physical realization of interlocked assemblies [14].
393 Contact was defined between all bodies with a stiff linear pressure-overclosure
394 relationship and a coefficient of friction of $\mu = 0.2$. A density $\rho = 0.95$ g/cm³
395 was considered and mass scaling by a factor of 100 was employed to reduce
396 computation time. 8-node reduced integration hexahedral elements (C3D8R)
397 were used to mesh the blocks, while solid 4-node tetrahedral elements (C3D4)
398 were used for the rigid frame. Enhanced hourglass control was used on the
399 hexahedral elements to reduce an observed tendency for hourgassing with
400 default hourglass control.

401 Mesh convergence was evaluated by comparing force-deflection data for
402 all models with mesh seed size over a range from $0.15 H_0$ to $0.21 H_0$ in in-
403 crements of $0.01 H_0$. It was found that convergent results for the computed
404 force-deflection behavior were obtained for almost all cases if the mesh seed
405 size is $0.16 H_0$. The exception to that finding were the [4⁴]-A and [4⁴]-B con-
406 figurations. These cases were susceptible to a perfect alignment of meshes
407 across contacts which tends to result in a nontraditional hourgassing across
408 contact interfaces. Such cross-contact hourgassing would create a local in-
409 terlocking feature between the blocks and prevent sliding. A seed size of
410 $0.17 H_0$ was used for the [4⁴]-A and [4⁴]-B configurations to avoid the mesh
411 alignment issue.

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