

Article

Wind Effects on Dome Structures and Evaluation of CFD Simulations through Wind Tunnel Testing

Tiantian Li ¹, Hongya Qu ², Yi Zhao ³, Ryan Honerkamp ³, Guirong Yan ^{3,*}, Arindam Chowdhury ⁴ and Ioannis Zisis ⁴ 

¹ Shanghai Typhoon Institute of China Meteorological Administration, Shanghai 200030, China

² Department of Bridge Engineering, Tongji University, Shanghai 200092, China

³ Department of Civil, Architectural and Environmental Engineering, Missouri University of Science and Technology, Rolla, MO 65409, USA

⁴ Department of Civil and Environmental Engineering, Florida International University, Miami, FL 33174, USA

* Correspondence: yang@mst.edu

Abstract: In the study, a series of wind tunnel tests were conducted to investigate wind effects acting on dome structures (1/60 scale) induced by straight-line winds at a Reynolds number in the order of 10^6 . Computational Fluid Dynamics (CFD) simulations were performed as well, including a Large Eddy Simulation (LES) and Reynolds-Averaged Navier–Stokes (RANS) simulation, and their performances were validated by a comparison with the wind tunnel testing data. It is concluded that wind loads generally increase with upstream wind velocities, and they are reduced over suburban terrain due to ground friction. The maximum positive pressure normally occurs near the base of the dome on the windward side caused by the stagnation area and divergence of streamlines. The minimum suction pressure occurs at the apex of the dome because of the blockage of the dome and convergence of streamlines. Suction force is the most significant among all wind loads, and special attention should be paid to the roof design for proper wind resistance. Numerical simulations also indicate that LES results match better with the wind tunnel testing in terms of the distribution pattern of the mean pressure coefficient on the dome surface and total suction force. The mean and root-mean-square errors of the meridian pressure coefficient associated with the LES are about 60% less than those associated with RANS results, and the error of suction force is about 40–70% less. Moreover, the LES is more accurate in predicting the location of boundary layer separation and reproducing the complex flow field behind the dome, and is superior in simulating vortex structures around the dome to further understand the unsteadiness and dynamics in the flow field.

Keywords: wind loads; Computational Fluid Dynamics simulation; wind tunnel testing; spherical domes; turbulence modeling



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1. Introduction

Spherical domes are commonly used as long-span space structures for public assembly venues, such as conference centers, concert halls, arenas, etc., since such a structure is at a distinct advantage due to high space usage and economic benefits. An example of a spherical building shown in Figure 1 is the Avicii Arena in Stockholm, Sweden. Its diameter is 110 m, and its inner height is 85 m. Winds can generate large loads on dome structures because of their large outer surface area, as well as severe vibrations because of their long-span and lightweight roofs. In fact, dome structures have suffered significant damage, and even total collapse, during past strong wind events. For example, the New Orleans Superdome suffered severe damage including the loss of its roof during Hurricane Katrina in 2005 (Figure 2a). The Reno/Virginia peak dome collapsed due to strong wind gusts in Western Nevada in 2008 (Figure 2b). Therefore, the wind resistance design of dome structures for structural failure elimination and safety assurance is necessary, and the aerodynamic behavior of dome structures and wind-induced loads need to be well understood.

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Figure 1. Avicii Arena in Stockholm, Sweden (by Stockholm Live).

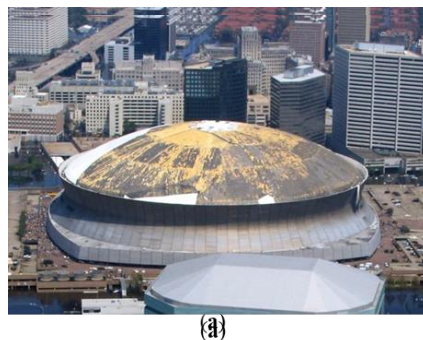


Figure 2. (a) The New Orleans Superdome badly damaged during Hurricane Katrina (by Trahan Architects/Asm Global); (b) The Reno/Virginia peak dome collapsed due to strong wind gusting (by NWS Reno, Nevada).

Wind tunnel tests were first conducted to explore mean [1,2] and fluctuating [3,4] wind pressures acting on dome structures. The impact of the Reynolds number (R_e) on smooth hemispherical domes was investigated, and the pressure at the dome apex monotonically increased with increasing R_e in the range of 3×10^4 to 8×10^4 [2]. However, for rough models, the pressure at the dome apex first increased and then decreased, and a maximum value was reached near the critical R_e of 5.7×10^4 [5]. The study of surface roughness effects on pressure distributions indicated that a smaller suction was induced at the dome apex and a larger suction in the wake region when the dome surface was rougher [6]. Cheng and Fu (2010) tested hemispherical domes with R_e ranging from 5×10^4 to 2.0×10^6 , and concluded that the pressure distribution became R_e -independent when it fell within 1.0 – 2.0×10^5 [7]. Aerodynamic loads on a semi-circular cylinder under various gust frequencies were studied, and a theoretical model was proposed to fit experimental results [8]. Wind tunnel tests of dome structures with various height-to-span ratios and opening ratios were conducted to investigate wind pressures on retractable dome roofs [9]. Mean pressure coefficients increased with the increase of the height-to-span ratio; additionally, as the opening ratio increased, the mean pressure coefficients at the outermost interior of open dome roofs increased. A series of measurements were carried out for an array of three domes, and the peak suction was observed approximately at the apex of the first dome, and the maximum positive pressure was observed on the windward side of the third dome [11].

Computational Fluid Dynamics (CFD) simulations are also employed to study aerodynamic behaviors of dome structures, which provide more details to understand flow characteristics regarding boundary layer separation. Turbulent flows are governed by the well-known Navier-Stokes (N-S) equations, which describe every detail of the turbulent flow field from the largest to the smallest length and time scales. Three types of numerical (RANS) simulation. The DNS directly solves N-S equations and resolves all scales of motion, and therefore, fine grid resolution down to the Kolmogorov scale is required. For example, the total number of grid points is estimated to be 10^{13} for a three-dimensional simulation of a turbulent flow with a Reynolds number of 10^6 [12]. Due to the high

computational demand of the DNS, its application to wind engineering is difficult and limited, especially for high Reynolds number problems. To reduce computational demand, the LES is developed to only resolve large scales of motion while smaller scales are removed from the flow variables by filtering the original N-S equations in the physical space. Smaller scales are then analyzed by subgrid-scale models, such as the Smagorinsky–Lilly model [13], the wall-adapting local eddy-viscosity (WALE) model [14], and the dynamic Smagorinsky–Lilly model [15,16].

In the RANS simulation, N-S equations are averaged over all scales of motion, and therefore, the turbulent flow is described by statistical parameters, e.g., mean velocity. The effects of turbulent fluctuations on the averaged flow are represented by additional terms obtained in the averaging process, which are called Reynolds stresses prescribed by turbulence models. Turbulence models are categorized according to the number of required transport equations. For a one-equation model, there is only one transport equation, and the Spalart–Allmaras (SA) model [17] is commonly used. The SA model shows the reasonably robust and time efficient capability to solve mildly separated flows. Additionally, it uses less memory and converges easily. For the two-equation model, there are two transport equations. The standard k - ε [18] and standard k - ω models [19] are commonly deployed. Turbulent kinetic energy is described by k , and the dissipation rate of k is described by ε and ω . Due to the fact that k at the impinging region is overestimated by the standard k - ε model when the airflow passes over an isolated obstacle [12], the realizable k - ε model [20] and renormalization group (RNG) k - ε model [21] are developed. The standard k - ω model is sensitive to inlet boundary conditions, and then the shear stress transport (SST) k - ω model [22–24] is developed, which predicts boundary layer separation and reattachment more accurately.

The LES and RANS simulations with the RNG k - ε turbulence model were applied to study velocity profiles in the wake of a dome [25,26]. The RNG k - ε model performed worse in simulating flow characteristics in the wake of the dome, and the LES showed better agreement with measurements. The LES was also applied to simulate mean pressures acting on a dome structure, which matched fairly well with experimental results [27]. Locations of boundary layer separation were predicted through the LES utilizing different subgrid-scale models, consisting of the Smagorinsky–Lilly model, the WALE model, and the dynamic Smagorinsky–Lilly model [28]. Since very fine grids were used in the LES, only minor deviations among different subgrid-scale models were observed. Wind pressures on scallop domes with a parabolic form of grooving were studied based on CFD simulations, and equations for surface pressure distribution were developed considering different height-to-span ratios [29]. Intensive CFD studies were conducted for dome structures to explain the capability of the CFD technique for determining appropriate design wind data [30]. It was concluded that pressure coefficients on the windward side and apex of the dome agreed well with corresponding Euro Code values, while pressure coefficients on the leeward side were different from all code standards. Wind loads acting on fifteen traditional domes were simulated by the CFD approach, and wind loads, especially suction force, were significantly affected by the shape of the dome [31].

The literature reviewed indicates that the DNS is able to provide every detail of the turbulent flow, but it is difficult to implement in flows of practical interest because of high computational demand. In comparison to the RANS simulation, the main properties of turbulent flows can generally be obtained by the LES with higher accuracy, but at a cost of higher memory usage and CPU time [32–34]. The accurate calculation of turbulent flow properties affects the reproduction of the flow structure of the wind field and hence, the accuracy of the wind pressure distribution on the dome surface. Considering respective advantages of the LES and RANS simulations, both of them are adopted in the current study. In the RANS simulation, the SA and SST k - ω turbulence models are selected because better prediction of boundary layer separation and reattachment can be achieved. Moreover, they can obtain relatively accurate results while reserving computational efficiency and good convergence. The LES is also employed because it is time-dependent and can

accuracy of the wind pressure distribution on the dome surface. Considering respective advantages of the LES and RANS simulations, both of them are adopted in the current study. In the RANS simulation, the SA and SST $k-\omega$ turbulence models are selected because better prediction of boundary layer separation and reattachment can be achieved. Moreover, they can obtain relatively accurate results while reserving computational efficiency and good convergence. The LES is also employed because it is time-dependent and can reproduce turbulence with much higher accuracy. For wind engineering applications, reproducing turbulence with much higher accuracy is required. For wind engineering applications, it is not only practical to compare most of the CFD simulation results to wind tunnel testing to validate their accuracy with the prediction of wind pressure distribution on the dome surface. Therefore, the wind tunnel testing of the dome structure is first performed in the NSF Natural Hazards Engineering Research Infrastructure (NHERI) Wall of Wind Experimental Facility (WOW EF) at Florida International University (FIU), and then it was employed as a benchmark to evaluate the RANS and LES simulations. The structure of the paper is as follows: First, the dome geometry and WOW EF are described, and the experimental setup is outlined. Second, experimental results are analyzed and discussed. Third, the LES and RANS simulations are applied to numerically simulate the wind tunnel testing, and then their performances are evaluated. Finally, conclusions are drawn.

2. Experimental Setup

This section consists of four parts. First, the geometry of the dome structure is described. Then, the WOW EF at FIU is briefly introduced. The experimental setup in the WOW EF is then followed. Finally, all testing cases are explained.

2.1. Dome Geometry

The apex height and base diameter of the prototype dome structure are 40 m and 120 m, respectively. A model scale of 1:60 is selected, and therefore, the scaled dome model has an apex height of $H \equiv 0.67$ m and a base diameter of $D \equiv 2.0$ m (Figure 3). The dome model is constructed with a smooth shell whose thickness is 9.0 mm. The blockage ratio, which is defined as the ratio of the projected area of the dome model and the test section of the WOW EF, is 0.7%. It is worth mentioning that the model scale of current testing is relatively large compared to previous studies; for example, the base diameters of the tested domes from other studies are mainly smaller than 1 m [14,5]. Such large-scale wind tunnel testing can improve the correspondence between wind tunnel testing results and actual pressure distribution on the prototype structure.

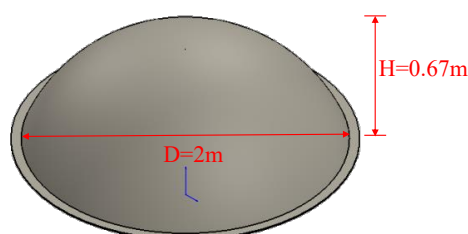


Figure 3. The dome model for wind tunnel testing.

2.2. Introduction of WOW EF

Figure 4 presents the WOW EF, which is a large wind tunnel with an open test section. As shown in Figure 4a, it is housed in a 30.5 m $\times$ 24.4 m $\times$ 10.7 m building with a gate of 19.24 m $\times$ 9.14 m. Wind-driven rain testing is allowed, and up to Category 5 wind speeds of 70.2 m/s can be generated by twelve fans at the South end. A small single-story building can be sufficiently immersed in the generated wind field, and thus, small structures can be tested at full scale to failure. The airflow travels through the flow conditioning section of 6.10 m $\times$ 4.27 m (Figure 4c), consisting of floor roughness elements and/or spires, to achieve target boundary layer characteristics and turbulence. A distance of 6 m far away from the target boundary section is the turning table, whose diameter is 4.9 m. The debris wall, which is used to stop wind-borne debris, is located 60 m to the North of the turning table center (Figure 4b).

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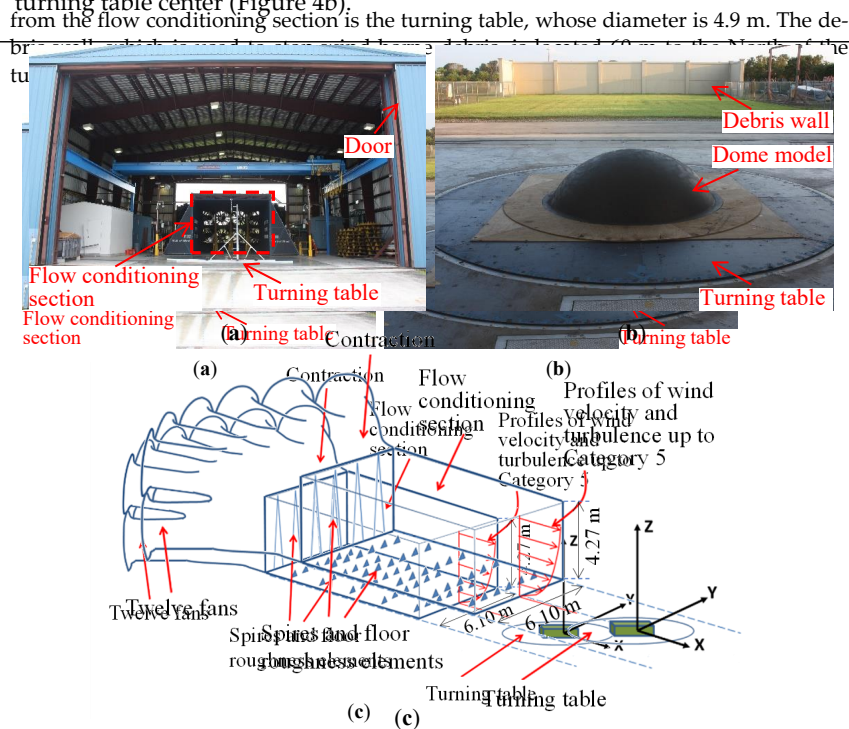


Figure 4. The WOW EF: (a) Flow management system and test section; (b) The model placed on top of the turning table; (c) Schematic diagram of the WOW EF.

2.3. Experimental Setup

The coordinate system origin is defined at the turning table center where the dome model is located (Figure 5). When the northern part of the dome model sits in the North, the angle (θ) of the turning table is defined as 0°. When the turning table rotates clockwise, the positive angle (θ) is defined as θ . Spires are used to generate open terrain configuration (Figure 6a), and both spires and surface roughness elements are used to generate suburban terrain configuration (Figure 6b). Spires are used to generate open terrain configuration (Figure 6a), and both spires and surface roughness elements are used to generate suburban terrain configuration (Figure 6b).

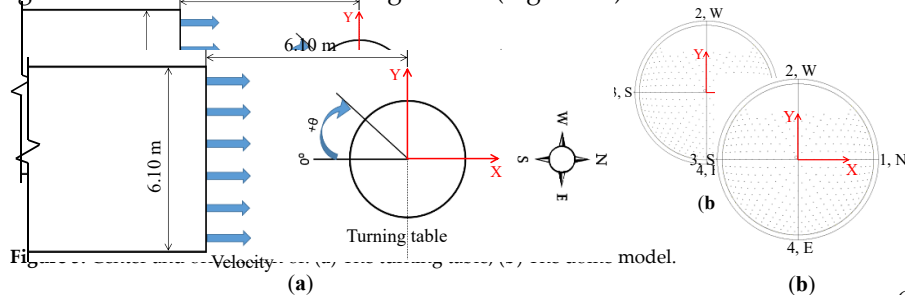


Figure 5. Center and orientation of: (a) The turning table; (b) The dome model.



Figure 6. (a) Open terrain configuration with spires; (b) Suburban terrain configuration with both spires and surface roughness elements.

The Cobra Probe manufactured by Turbulent Flow Instrumentation is used to measure the flow field at a frequency of 2500 Hz. It is a four-hole pressure probe, and resolves three components of velocity and local static pressure in real-time. The local coordinate system of the Cobra Probe is illustrated in Figure 7, which is used to calculate velocity components of each probe.

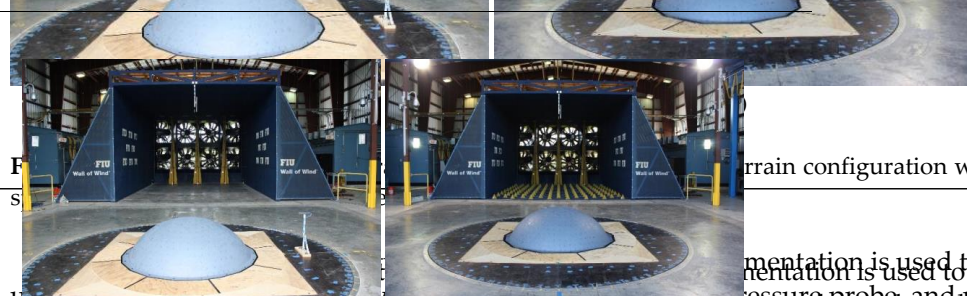


Figure 6.

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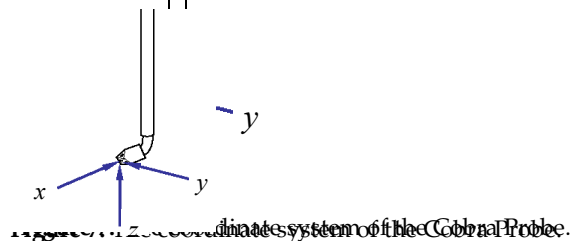


Figure 7. The coordinate system of the Cobra Probe.

The Cobra Probes are installed at different heights of 16.5 cm, 33.3 cm, 66.7 cm, and 133.4 cm to measure wind velocity profiles at the turning table center ($X = 0$ and $Y = 0$). In this case, the dome is removed from the wind tunnel (Figure 8) and the measurement is only made for open terrain configuration.

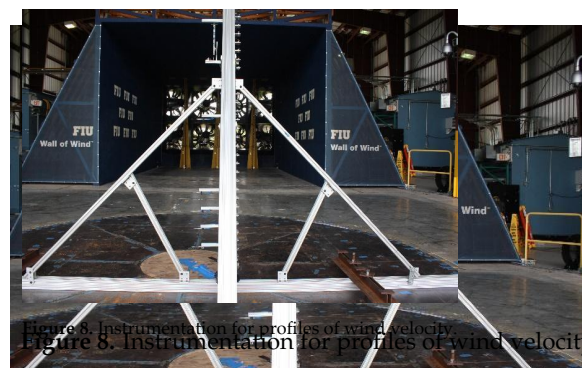


Figure 8. Instrumentation for profiles of wind velocity.

Figure 9 presents the arrangement of 384 pressure taps on the dome model, which are used to measure the surface pressure of the dome model. A small region in the positive Y direction is not covered because of the availability of pressure taps. Scanivalve ZOC33 electronic pressure scanners are deployed for data collection at frequencies of 520 Hz. An amount of 64 piezoresistive pressure sensors with a range of ± 0.254 m H₂O are incorporated in each ZOC33 module, which are connected to pressure taps with PVC 105-16 tubing. The PVC tubing is 0.3 m long on the side of the tap hole, and 0.6 m long on the side of the sensor.

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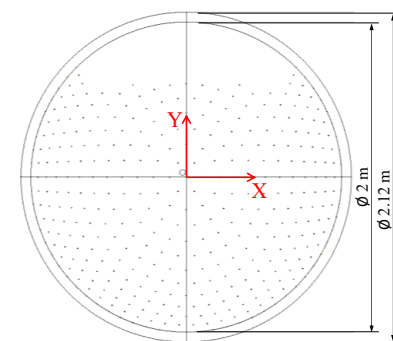


Figure 9. Pressure taps installed on the dome model.

2.4. Testing Cases

Three different cases for the wind tunnel testing were conducted (Table 1). In Case 1, open terrain configuration is considered for the wind field and the dome is removed. In Cases 2–3, the dome is placed in the wind tunnel. The open terrain is employed in Case 2, while the suburban terrain is employed in Case 3. Within each case, three different levels of velocity are generated, i.e., 24 m/s (Level I), 34 m/s (Level II), and 44 m/s (Level III) at the height of 0.167 m (corresponding to 10 m in full scale).

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Table 1. Cases for the wind tunnel testing.

	Case 1			Case 2			Case 3		
Dome model present?	No			Yes			Yes		
Terrain configuration	Open			Open			Suburban		
Wind speed level	I	II	III	I	II	III	I	II	III

3. Experimental Results and Discussion

3.1. Mean Velocity Profile

Profiles of mean wind velocity measured over the open terrain without the dome (Case 1) are depicted in Figure 10. Three different levels of velocity are generated, which are specified by the wind speed at the height of 0.167 m, 24 m/s (Level I), 34 m/s (Level II), and 44 m/s (Level III). To reduce measurement uncertainties, the first and last 10 s of each velocity time history are removed, and then the mean wind velocity is determined based on the middle range of the record. As the elevation increases, the mean velocity increases. The power law is used to fit mean velocity profiles, and the applied exponent is 9.5 for the Open terrain (Surface Roughness C) based on ASCE 7–16 [35]. The coefficients of determination (R^2) of the power law fit for velocity Level I, Level II, and Level III are 0.75, 0.88, and 0.91, respectively. The mean velocity profile at Level III agrees with the power law the best.

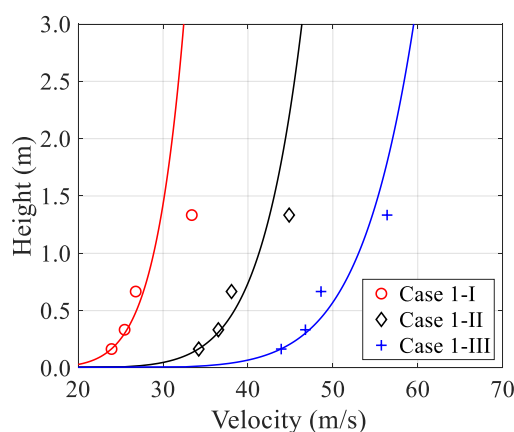


Figure 10. Profiles of mean along-wind velocity for open terrain without the dome (Case 1).

3.2. Turbulence Intensity and Integral Length Scale

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Turbulence of the airflow is characterized by a scale expressed as a percentage, that is, turbulence intensity (TI). It is defined as the ratio of root-mean-square of the fluctuating wind velocity to the mean velocity, according to the measured velocity time series (Equation (1)).

$$I = \frac{u'}{U} \quad (1)$$

$$u' = \sqrt{\frac{1}{3}(u_x'^2 + u_y'^2 + u_z'^2)}$$

$$U = \sqrt{U_x^2 + U_y^2 + U_z^2}$$

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$$U = \sqrt{U_x^2 + U_y^2 + U_z^2}$$

where u' is the root-mean-square of the fluctuating components of the measured velocity. U is the mean wind velocity over the same time period of the measured velocity component. U_x , U_y , and U_z are the mean wind velocity over the same time period of the measured velocity component, indicated by subscripts X, Y, and Z. Figure 11 shows the TI for Case 1 over the open terrain, based on Equation (1). The TI slightly decreases with the increasing height. In general, they are all around 10% at different heights.

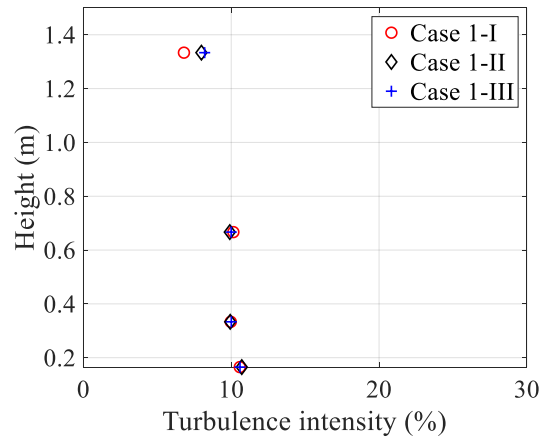


Figure 11. Turbulence intensity (TI) along height over open terrain (Case 1).

The integral length scale of turbulence is a measure of the average size of turbulent eddies. There is a total of nine length scales, representing three dimensions of the eddies associated with the flow. For the longitudinal direction, the average longitudinal components of the eddies are related to the longitudinal velocity fluctuations, as defined in Equation (2) based on Taylor's hypothesis and the assumption that the flow disturbance travels with the velocity U_x [37]. The calculated L_x^u is presented in Figure 12, which ranges from 0.5 m to 0.7 m. L_x^u is more randomly distributed, since no obvious tendency is found along height or among different cases.

$$L_x^u = \frac{U_x}{u_x'^2} \int_0^\infty R(\tau) d\tau \quad (2)$$

where $R(\tau)$ is the autocovariance function of the longitudinal velocity fluctuations.

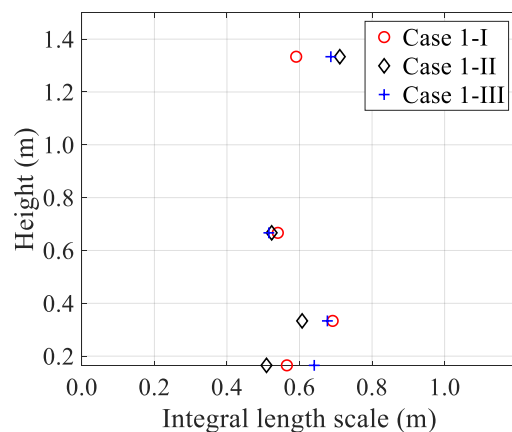
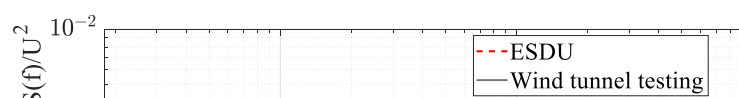


Figure 12. Integral length scale along height over open terrain (Case 1).

3.3. Power Spectral Density

Figure 13 presents the power spectral density (PSD) of velocity in the along-wind direction. Figure 13 represents the power spectral density (PSD) of velocity in the along-wind direction at a height of 0.7 m (at the apex) for Case 1-III over the open terrain. The measured PSD agrees fairly well with the ESDU spectrum, except at around 10 Hz where the experimental data are higher.



3.3. Power Spectral Density

Figure 13 presents the power spectral density (PSD) of velocity in the along-wind direction at a height of 0.67 m (dome apex) for Case 1-III over the open terrain. The measured PSD agrees fairly well with the ESDU spectrum, except at around 10 Hz where the experimental data are higher.

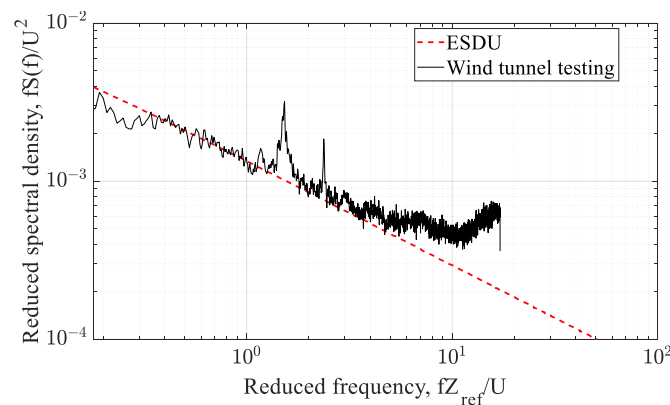


Figure 13. Power spectral density (PSD) of velocity in the along-wind direction at a height of 0.67 m (dome apex) over open terrain (Case 1-III).

3.4. Reynolds Number

The Reynolds number is the ratio of inertial to viscous forces, which is defined as

$$Re = \frac{\rho V H}{\mu} \quad (3)$$

where ρ is the density of air (1.16 kg/m^3), and it is determined based on the air temperature (30°C), humidity (65%), and atmospheric pressure (101 kPa) at the time of the test; H is the apex height of the dome (0.67 m); V is the reference mean velocity at H (27 m/s, 38 m/s, and 49 m/s from Case 1 over the open terrain for respective velocity level); μ is dynamic viscosity of air when the air temperature is 30°C ($1.86 \times 10^{-5} \text{ N}\cdot\text{s/m}^2$). Based on Equation (3), the Reynolds numbers for three levels of velocity in Case 1 without the dome model are determined as 1.13×10^6 , 1.59×10^6 , and 2.05×10^6 , respectively.

3.5. Mean Pressure Coefficient on the Center Meridian of the Model

The dimensionless number, the pressure coefficient (C_p), is used to describe the relative wind pressure acting on the dome model. It is normalized by the dynamic pressure at the apex of the dome as:

$$C_{P_i} = \frac{P_i - P_{ref}}{\frac{1}{2}\rho V^2} \quad (4)$$

where $P_i - P_{ref}$ is measured by the Scanivalve instrumentation, indicating the pressure difference between the local and reference pressure P_{ref} ; V is the reference mean velocity, which is the same as in Equation (3). In cases 2–3, the V from each wind speed level is used to calculate the pressure coefficient under the associated case, i.e., 27 m/s is used for wind speed level I, 38 m/s for wind speed level II, and 49 m/s for wind speed level III.

Mean pressure coefficients (C_p) on the center meridian of the dome model are presented in Figure 14. For all Cases, the profiles are of a similar shape, which are consistent with other reported studies [1,2,5,7]. The maximum pressure occurs at an elevation of about 20 degrees, and the minimum pressure appears at around 90 degrees. At around 45 degrees, the positive pressure changes to negative pressure. In addition, the pressure coefficient generally increases with inflow wind velocity, and the magnitude of the pressure coefficient over the suburban terrain is lower than that over the open terrain.

The results associated with Case 2-III are further compared to other studies with similar Reynolds numbers using smooth hemispheres in boundary layer flows [1,7]. Figure 15 indicates that the general trends among different studies are similar. A minor difference is found regarding the peak values of positive and negative pressure. At around 120 degrees, the curve of the present study is relatively different from the other two studies, implying that the pressure distribution on the leeward side of the dome may be different. This is

coefficient over the suburban terrain is lower than that over the open terrain. The results associated with Case 2-III are further compared to other studies with similar Reynolds numbers using smooth hemispheres in boundary layer flows [1,7]. Figure 15 indicates that the general trends among different studies are similar. A minor difference is found regarding the peak values of positive and negative pressure. At around 120 degrees, the curve of the present study is relatively different from the other two studies, implying that the pressure distribution on the leeward side of the dome may be different. This is probably due to different ratios of the model height to the diameter, which is 0.335 in the present study and is 0.5 for the other two studies.

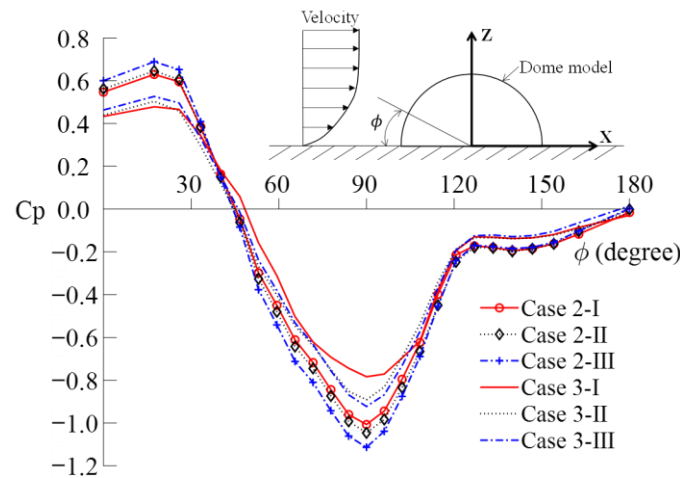


Figure 14. Comparison of pressure coefficient (C_p) on the center meridian of the dome model.

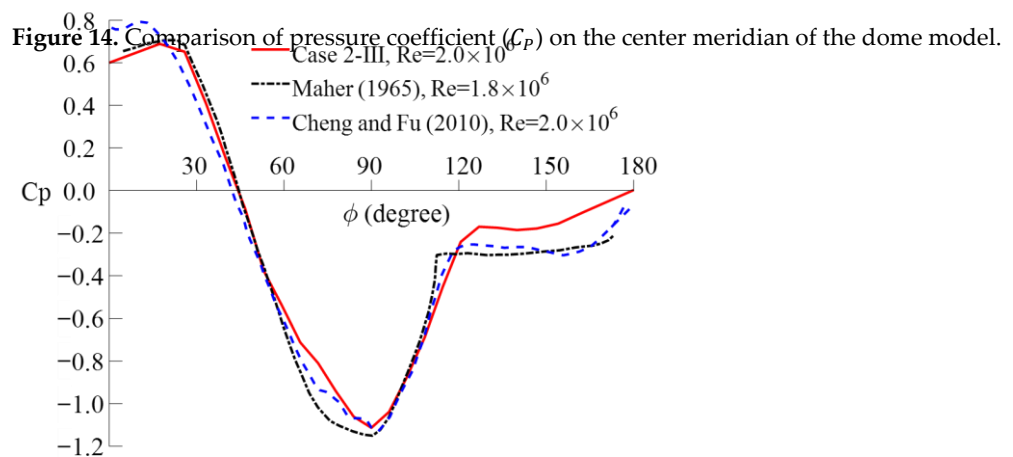


Figure 15. Comparison of pressure coefficient (C_p) on the center meridian of the dome model with other studies [1,7].

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3.6. Wind Pressure Distribution on Dome Surface

3.6.1. Open Terrain

Figure 16 shows the wind pressure distribution on the dome surface over the open terrain. For each velocity level, the wind velocity ramps from 0% to the target wind velocity, and then keeps constant for 60 s. The mean pressure coefficient is calculated according to time averaging of the middle 40 s (10–50 s) from the last 60 s dataset, in order to reduce measurement uncertainties. As illustrated in Figure 9, a small top region of the dome surface in the positive Y direction is not covered by pressure taps, and the associated wind pressure distribution is unknown. To tackle this problem, the wind pressure of the unmeasured region is mapped from the corresponding region in the negative Y direction. Figure 16 indicates that the patterns of pressure coefficients are similar for all three velocity levels. Positive pressure occurs on the windward side and negative (suction) pressure occurs at the leeward side. The maximum positive pressure occurs near the base of the dome, which is around 0.6. Negative pressure decreases when moving from the base of the dome to the apex of the dome, and the minimum (suction) pressure coefficient occurs over the apex of the dome, which is around -1.0 . Then, negative pressure increases towards the leeward edge, and the negative pressure coefficient near the base of the dome on the leeward side is around -0.2 . Among the three velocity levels, the area enclosed by the isoline of 0.6 near the base increases with the increase of wind speed. For the area enclosed by the isoline of -1.0 , only a small area appears for

enclosed by the isoline of 0.6 near the base increases with the increase of wind speed. For the area enclosed by the isoline of -1.0 , only a small area appears for Case 2-I, a slightly larger area for Case 2-II, and a very large area for Case 2-III. This indicates that when the wind speed increases, the larger region of the dome surface is exposed to extreme negative or suction wind pressure, implying that the dome withstands greater wind forces.

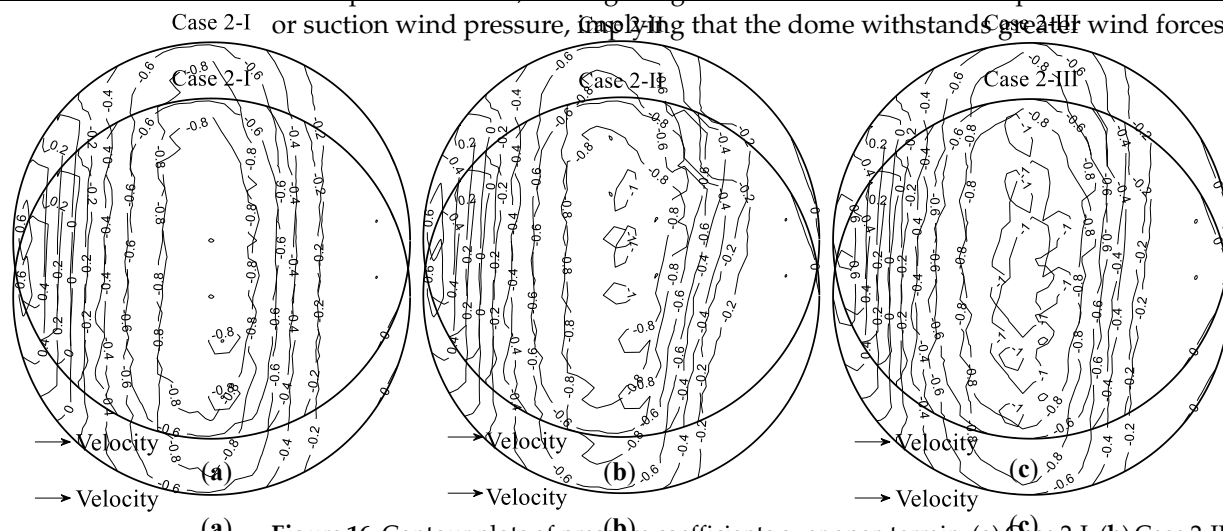


Figure 16. Contour plots of pressure coefficients over open terrain: (a) Case 2-I; (b) Case 2-II; (c) Case 2-III.

Figure 16. Contour plots of pressure coefficients over open terrain: (a) Case 2-I; (b) Case 2-II; (c) Case 2-III.

3.6.2. Suburban Terrain
Figure 17 presents the wind pressure distribution over the suburban terrain. The same strategies of wind tunnel testing and data processing are used for the suburban terrain as those for the open terrain. Compared to Figure 16, a similar pattern of wind pressure distribution is found for the two different terrains. However, lower absolute values of both positive and negative pressure coefficients are encountered for the suburban terrain. This is probably caused by higher surface roughness resulting from the suburban terrain.

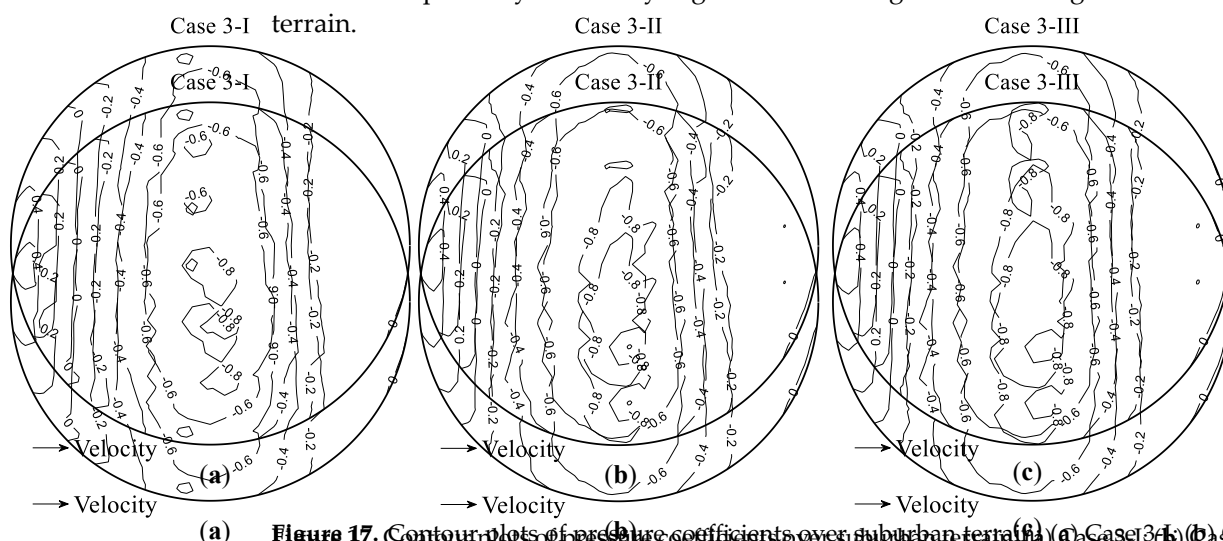


Figure 17. Contour plots of pressure coefficients over suburban terrain: (a) Case 3-I; (b) Case 3-II; (c) Case 3-III.

Figure 17. Contour plots of pressure coefficients over suburban terrain: (a) Case 3-I; (b) Case 3-II; (c) Case 3-III.

3.7. Force and Moment Coefficient

All forces and moments exerted onto the dome are calculated based on the integration of the surface pressure. Table 2 lists calculated forces and moments for both terrains. The most significant wind load is suction force (F_z). Because the dome and wind pressure distribution on the dome surface are symmetric, the overturning moment M_x , the rotational moment M_z , and the drag force F_y are very small, which can be neglected. In terms of F_x and M_y , they generally increase when the wind velocity increases.

moment M_Z , and the drag force F_Y are very small, which can be neglected. In terms of F_X and M_Y , they generally increase when the wind velocity increases.

Table 2. Force and moment.

	Open Terrain			Suburban Terrain		
	Case 2-I	Case 2-II	Case 2-III	Case 3-I	Case 3-II	Case 3-III
F_X (N)	45.9	74.1	136.3	59.7	76.4	127.4
F_Y (N)	−2.6	16.5	1.1	−9.6	−19.6	−23.5
F_Z (N)	597.2	1188.4	2151.8	479.2	1056.0	1764.8
M_X (N·m)	−1.1	6.9	0.4	−4.0	−8.2	−9.8
M_Y (N·m)	−19.1	−31.0	−57.0	−24.9	−31.9	−53.2
M_Z (N·m)	−0.02	−0.03	−0.08	−0.03	−0.05	−0.08

Force and moment coefficients are also compared, which are normalized by the dynamic pressure as defined in Equation (5). Table 3 lists the calculated force and moment coefficients. The suction force coefficient is also the largest among all coefficients. However, the difference of the suction force coefficient among different velocity levels is smaller because of normalization. For example, the difference of the suction force coefficient between Case 2-I and Case 2-III is around 9%, but such a difference of suction force is around 72%.

$$\begin{aligned}
 C_{F_X} &= \frac{F_X}{\frac{1}{2}\rho V^2 A}, \quad C_{M_X} = \frac{M_X}{\frac{1}{2}\rho V^2 A H} \\
 C_{F_Y} &= \frac{F_Y}{\frac{1}{2}\rho V^2 A}, \quad C_{M_Y} = \frac{M_Y}{\frac{1}{2}\rho V^2 A H} \\
 C_{F_Z} &= \frac{F_Z}{\frac{1}{2}\rho V^2 A}, \quad C_{M_Z} = \frac{M_Z}{\frac{1}{2}\rho V^2 A H}
 \end{aligned} \quad (5)$$

where F_X , F_Y , and F_Z are forces exerted onto the dome; M_X , M_Y , and M_Z are moments exerted onto the dome; H is the height of the dome apex (0.67 m); A is the projected area of the dome model on a plane parallel to the cross-section of the wind tunnel (0.97 m²); V is the reference mean velocity at the dome apex (27 m/s, 38 m/s, and 49 m/s from Case 1 for three velocity levels).

Table 3. Force coefficient and moment coefficient.

	Open Terrain			Suburban Terrain		
	Case 2-I	Case 2-II	Case 2-III	Case 3-I	Case 3-II	Case 3-III
C_{F_X}	0.11	0.09	0.10	0.15	0.09	0.09
C_{F_Y}	−0.01	0.02	0.00	−0.02	−0.02	−0.02
C_{F_Z}	1.46	1.46	1.59	1.17	1.30	1.31
C_{M_X}	0.00	0.01	0.00	−0.01	−0.02	−0.01
C_{M_Y}	−0.07	−0.06	−0.06	−0.09	−0.06	−0.06
C_{M_Z}	0.00	0.00	0.00	0.00	0.00	0.00

4. Numerical Simulations and Discussion of Results

In the study, the commercial code Fluent 19.2 is utilized to numerically simulate the wind tunnel test. The profile of the mean velocity measured at the turning table center without the dome model is used as the velocity input. Since the profile of the mean velocity is measured over the open terrain, only the wind tunnel test over the open terrain with maximum wind velocity is numerically simulated, that is, Case 2-III. In addition, the Reynolds numbers of the numerical simulation and the wind tunnel test are the same.

4.1. Numerical Simulations of Wind Tunnel Testing

4.1.1. Numerical Model

Figure 18 presents the numerical model used for CFD simulations. The wind tunnel, from the test section to the debris wall, is numerically simulated. Part A of the numerical

ical model is 66.10 m long, 15.24 m wide, and 9.14 m high, and the dimensions are the same as those of the wind tunnel. To investigate wind loads exerted onto the dome model, it is positioned at the same location as the turning table center.

The velocity inlet is set up on the left surface of Part A, which represents the cross section of the flow conditioning section. Apart from the velocity inlet, the remaining area of the left surface is defined as the no-slip wall. The boundary conditions of the front, top, and back surfaces of Part A are also specified as the no-slip wall. Regarding the front, top, and back surfaces of Part B, the pressure outlet is specified, since these regions are open to air. The no-slip wall is also applied to the ground, the dome surface, and the debris wall. The shaded areas in Figure 18 indicate the boundary conditions of the no-slip wall, excluding the ground. To be consistent with the wind tunnel test, the turning table center, 6.1 m away from the velocity inlet, is defined as the coordinate system origin.

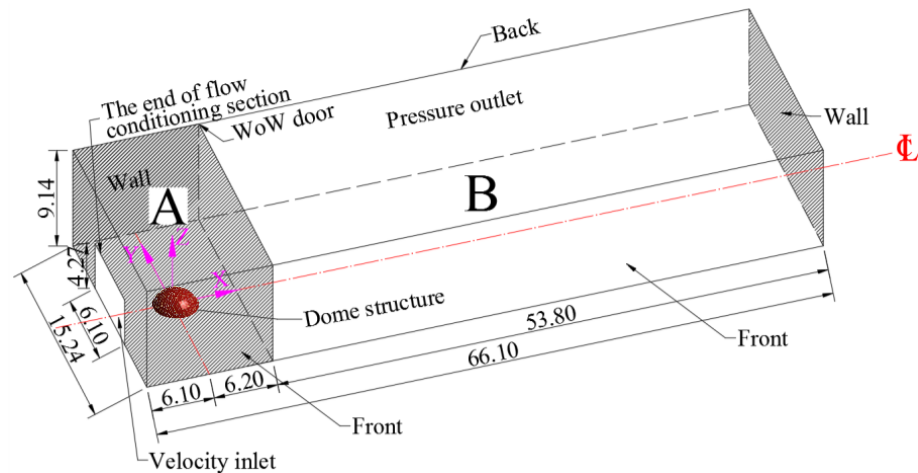


Figure 18. Numerical model of CFD simulations (unit: m).

The velocity inlet is set up on the left surface of Part A, which represents the cross section of the flow conditioning section. Apart from the velocity inlet, the remaining area of the left surface is defined as the no-slip wall. The boundary conditions of the front, top, and back surfaces of Part A are also specified as the no-slip wall. Regarding the front, top, and back surfaces of Part B, the pressure outlet is specified, since these regions are open to air. The no-slip wall is also applied to the ground, the dome surface, and the debris wall. The shaded areas in Figure 18 indicate the boundary conditions of the no-slip wall, excluding the ground. To be consistent with the wind tunnel test, the turning table center, 6.1 m away from the velocity inlet, is defined as the coordinate system origin.

Turbulent flow is simulated using the Kolmogorov scale [38] for the RANS and LES. Equation (6) is applied to find the optimal meshing sizes for the RANS and LES. The numerical model is discretized into structured mesh, except the block containing the dome where unstructured mesh is used. To accurately calculate velocity gradients in the boundary layer, the inflation grid technique is utilized for the lowest layers near the ground and dome surface. Due to the inherent difference between the RANS and LES, different meshing sizes are applied. According to Section 3.2, the integral length scale for the turbulent flow is around 0.6 m. Moreover, the Kolmogorov scale [38] is estimated from Equation (6) as in the order of 10^{-5} m. In reference to both length scales, the grid independence study is performed to find the optimal meshing sizes for the RANS and LES. Consequently, the minimum size of the cells for the RANS and LES are 0.3 m and 0.001 m, respectively. The wall Y^+ values for the RANS and LES are 140 and 15, respectively. The total numbers of the generated cells for the RANS and LES are 0.43 million and 3.12 million, respectively.

$$\eta = \left(\frac{\nu^3}{\varepsilon} \right)^{1/4} \quad (6)$$

where ν is the kinematic viscosity, which is defined as the ratio of the dynamic viscosity of air (μ) over the density of air (ρ); ε is the rate of dissipation, which is scaled as U^3/L (U is the mean velocity and L is the integral length scale).

where ν is the kinematic viscosity, which is defined as the ratio of the dynamic viscosity of air (μ) over the density of air (ρ); ε is the rate of dissipation, which is scaled as U^3/L^2 (U is the mean velocity and L is the integral length scale).

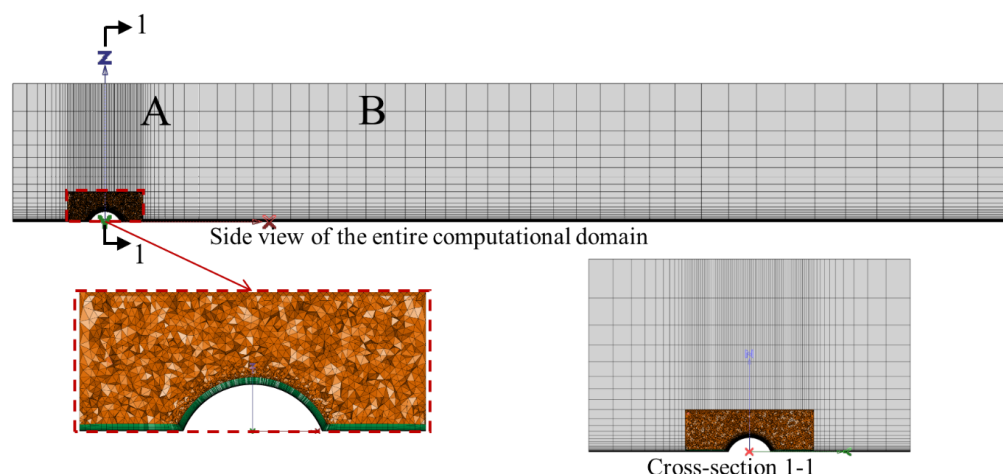


Figure 19. Grid arrangement of the numerical model for RANS (A and B represent Part A and B in Figure 18; X and Z represent axes of local coordinate system).

4.1.2. Determination of Velocity Input

4.1.2. Determination of Velocity Input

The measured mean velocity shown in Figure 10 is used to determine the velocity input at the flow inlet. Velocity measurements were made over the open terrain without the dome, and they were measured at the turning table center. The regression technique is applied to the along-wind velocity from Case 1-III. The applied regression equation is given in Equation (7), which is based on the power law [36].

$$\bar{V}(z) = \bar{V}(z_{ref}) \left(\frac{z}{z_{ref}} \right)^{1/\alpha} \quad (7)$$

where z is the height (m); z_{ref} is the reference height of 0.167 m, corresponding to 10 m in full scale; $\bar{V}(z_{ref})$ is the reference mean velocity at z_{ref} , that is, 44 m/s at 0.167 m in the full scale; $\bar{V}$ (direction) is the reference mean velocity at which is related to surface roughness and wind direction; α is the power-law exponent, which is related to surface roughness and Equation (8) calculated by determining regression equation, and α is determined as 9.47. The Equation (8) gives the determined regression equation, the comparison between the regression equation and experimental results. Figure 19a shows the comparison between the regression equation and experimental results. The thin solid line shows the measured velocity at the turning table center is (Figure 4). On the other hand, Equation (8) is applied at the velocity inlet in CFD simulations. To demonstrate this assumption, numerical simulations are performed when the dome model is removed from the numerical model, and the obtained results are also presented in Figure 20a. Velocity profiles extracted at 2 m from the inlet, 4 m from the inlet, and the center of the turning table are nearly the same as the velocity input, with the maximum mean error of 2.0 m/s and the minimum R^2 of 0.94, which are reasonable and acceptable. Profiles of TI at different locations (2 m from the inlet, 4 m from the inlet, and the turning table center) are also compared to experimental results in Figure 20b, where stronger fluctuations are encountered than velocity profiles. In general, the TI decreases as the height increases. Below the height of 1.5 m, numerically simulated TIs are close to experimental results, with the maximum mean error of 0.6%. It implies that similar levels of turbulence are achieved in CFD simulations compared with the wind tunnel testing.

$$V_{alongwind} = 44.4 \left(\frac{z}{0.167} \right)^{1/9.47} \quad (8)$$

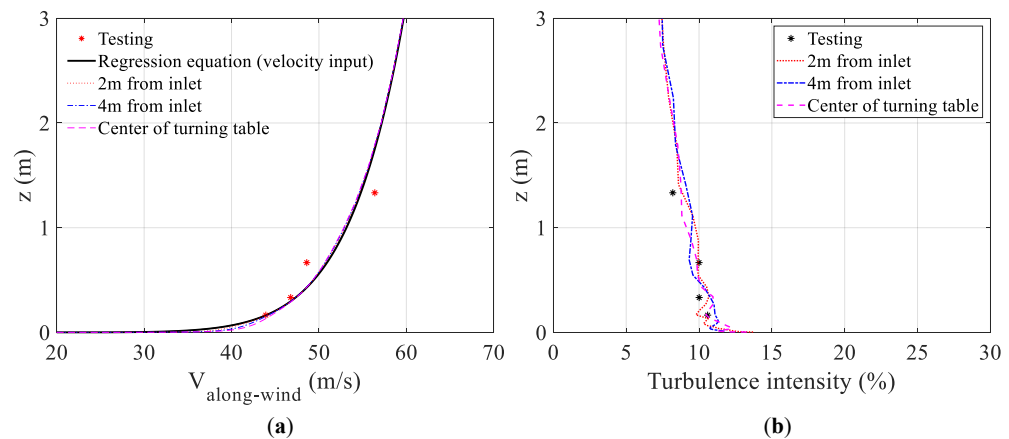


Figure 20. Comparison of (a) Velocity profiles; (b) Turbulence intensity profiles.

4.1.3. CFD Simulation Setup

Both the RANS and LES approaches are applied to analyze the turbulent flows. The RANS simulation provides time-averaged solutions to the N-S equations, which are presented in Equation (9).

$$\frac{\partial \langle u_i \rangle}{\partial t} + \langle u_j \rangle \frac{\partial \langle u_i \rangle}{\partial x_j} = -\frac{1}{\rho} \frac{\partial \langle p \rangle}{\partial x_i} + \nu \frac{\partial^2 \langle u_i \rangle}{\partial x_j \partial x_j} + \frac{\partial \langle -u_i' u_j' \rangle}{\partial x_j} \quad (9)$$

$$\frac{\partial \langle u_i \rangle}{\partial x_i} = 0 \quad (10)$$

where ρ is the air density, ν is the kinematic viscosity, $\langle u_i \rangle$ and $\langle u_j \rangle$ are time-averaged components of the instantaneous velocity u_i and u_j , and $\langle -u_i' u_j' \rangle$ is the Reynolds stresses, which arises in the averaging process and represents the effects of turbulence. Reynolds stresses require additional modelling to close the RANS equations. The SA and SST $k-\omega$ models are employed in the current study. They are all based on the Boussinesq hypothesis that relates the Reynolds stresses to the mean velocity gradients. The SA model only needs one additional transport equation that represents turbulent viscosity. The SST $k-\omega$ model involves two additional transport equations for turbulence kinetic energy k and the specific dissipation rate ω , and turbulent viscosity is calculated as a function of k and ω .

In the LES, large scales of motion are directly resolved, while the smaller scales are ignored by filtering the N-S equations. The governing equations for the LES are:

$$\frac{\partial \bar{u}_i}{\partial t} + \frac{\partial \bar{u}_i \bar{u}_j}{\partial x_j} = -\frac{1}{\rho} \frac{\partial \bar{p}}{\partial x_i} + \nu \frac{\partial^2 \bar{u}_i}{\partial x_j \partial x_j} + \frac{\partial \bar{\tau}_{ij}}{\partial x_j}; \quad \bar{\tau}_{ij} = \bar{u}_i \bar{u}_j - \bar{u}_i' \bar{u}_j'$$

$$\frac{\partial \bar{u}_i}{\partial x_i} = 0 \quad (10)$$

where $\bar{u}_i$ and $\bar{u}_j$ are filtered velocities; $\bar{p}$ is filtered pressure. $\bar{\tau}_{ij}$ is referred to as the sub-grid scale stresses, which are modeled and require additional modelling. The Smagorinsky-Lilly model is selected as the sub-grid scale model in the current study. The Smagorinsky constant C_s ranges from 0.1 to 0.23, and 0.1 is used here.

For the RANS simulations, a steady RANS is performed instead of an unsteady RANS (URANS). The URANS requires a high spatial resolution, and it is recommended to directly use the LES or hybrid URAN/LES, especially for atmospheric boundary layers with relatively high turbulence [39,40]. Moreover, the URANS only simulates statistics and does not simulate turbulence. Since the LES is applied in the current study, the URANS is not considered here. In the RANS simulations, the coupled solver combined with the pseudo transient mechanism is selected, and the second order discretization scheme is used for momentum and continuity equations. In the LES, the semi-implicit method for the

with the pseudo transient mechanism is selected, and the second order discretization scheme is used for momentum and continuity equations. In the LES, the semi-implicit method for the pressure linked equation-consistent (SIMPLEC) as a segregated solver is used. The bounded central differencing discretization scheme is used for momentum equations, second order discretization is used for pressure, and the bounded second order implicit for transient formulation is used. The time step of the LES is 0.0001 s. Such a small time step is used to accurately capture sufficient information from the simulation and to achieve the converged solution. Residuals of velocity, continuity, and other parameters are monitored to decide the convergence of the solution, together with quantitative validation. Residuals values of velocity, pressure, and other parameters are decided to be 0.0001, and all residuals stopping below the tolerance are a sign of convergence. Values of all large quantities are varied. The tolerance of the residuals is 0.001, and conditions for stopping below the tolerance are decided. Moreover, all target quantitative variables are monitored to reach relatively stable conditions for the achievement of the converged solution.

4.2. Discussion of Results

4.2.1. Mean Pressure Coefficient

4.2.1. Mean Pressure Coefficient

The distributions of the mean pressure coefficients on the center meridian of the dome for the three cases associated with the SA model, SST $k-\omega$ model, and the LES are presented in Figure 21. The minimum pressure occurs at an elevation of about 90 degrees, and the maximum pressure occurs at around 20 degrees. The general trends of numerical results along the meridian agree with the testing data. However, the LES result reaches the minimum pressure slightly earlier than reaching the meridian angle of 90 degrees, while for the RANS results, later than 90 degrees. The distribution of pressure coefficients minimum pressure slightly earlier than reaching the meridian angle of 90 degrees, while for the RANS results, later than 90 degrees. The distribution of pressure coefficients along the meridian for the RANS results shifts slightly to the right of the testing data, exhibiting a discrepancy between the RANS results and the testing data. At around 120 degrees, the negative pressure gradually changes to a stable condition, and the LES result indicates a similar tendency. However, a relatively larger difference exists between the RANS results and the testing data. It is possible that the RANS simulation performs worse than the LES in simulating boundary layer separation and reattachment on the leeward side. The mean error and root-mean-square error are used to quantify the disagreement between numerical and testing results. Mean errors associated with the SA, SST $k-\omega$ model, and the LES are 0.12, 0.10, and 0.04, respectively, while the root-mean-square errors are 0.15, 0.13, and 0.05, respectively. Overall, errors associated with the LES are much smaller than the RANS results, regarding the distribution of the pressure coefficient along the center meridian.

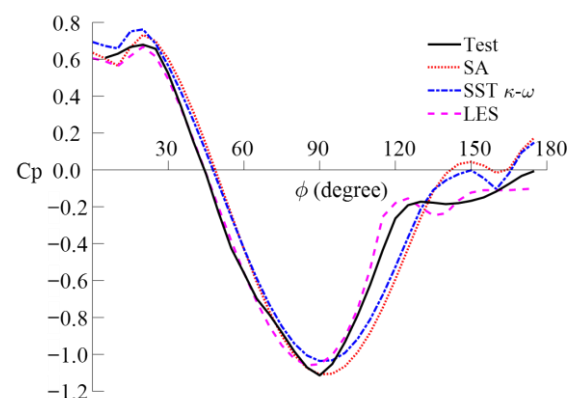


Figure 21. Comparison of pressure coefficients on the center meridian of the dome model between numerical simulations and the wind tunnel testing.

The distributions of the mean pressure coefficient from the CFD simulations are shown in Figure 22. The overall patterns from all cases could match with the wind tunnel testing, that is, the wind pressure is positive near the base of the dome on the windward side, negative pressure is observed on the leeward side, and the highest intensity of negative pressure occurs at the apex of the dome. The isolines of numerical results are smoother than the experimental results, which is probably caused by finer grids for data processing and certain idealization of numerical simulations. Among the three cases, the isolines from the

shown in Figure 22. The overall patterns from all cases closely match with wind tunnel testing, that is, the wind pressure is positive near the base of the dome on the windward side, negative pressure is observed on the leeward side, and the highest intensity of negative pressure occurs at the apex of the dome. The isolines of numerical results are smoother than the experimental results, which is probably caused by finer grids for data processing and certain idealization of numerical simulations. Among the three cases, the isolines from the LES results agree with the wind tunnel testing the best, and only minor discrepancies are found near the base of the dome on the leeward side. For the results associated with the SA and SST $k-\omega$ models, the isolines of positive pressure seem to shift to the right of those from the experimental results, and a relatively large difference is observed for the isolines at the dome apex and on the leeward side. The numerically simulated forces and moments are compared to the experimental results as listed in Table 4. As stated in Section 3.7, M_x , M_y and F_z are small enough to be neglected due to the symmetry of the dome structure and grouted loads. Therefore, only F_x and M_y are compared. Errors of F_x are observed with the RANS simulations range from 2% associated with the RANS and LES range from 9.3% to 17.2%, and errors of the LES results are much smaller than the RANS results. The errors of M_y are much smaller than the RANS results and M_y are 6.6% and 5.2%, respectively. Errors of F_x and M_y are 6.6% and 5.2%, respectively.

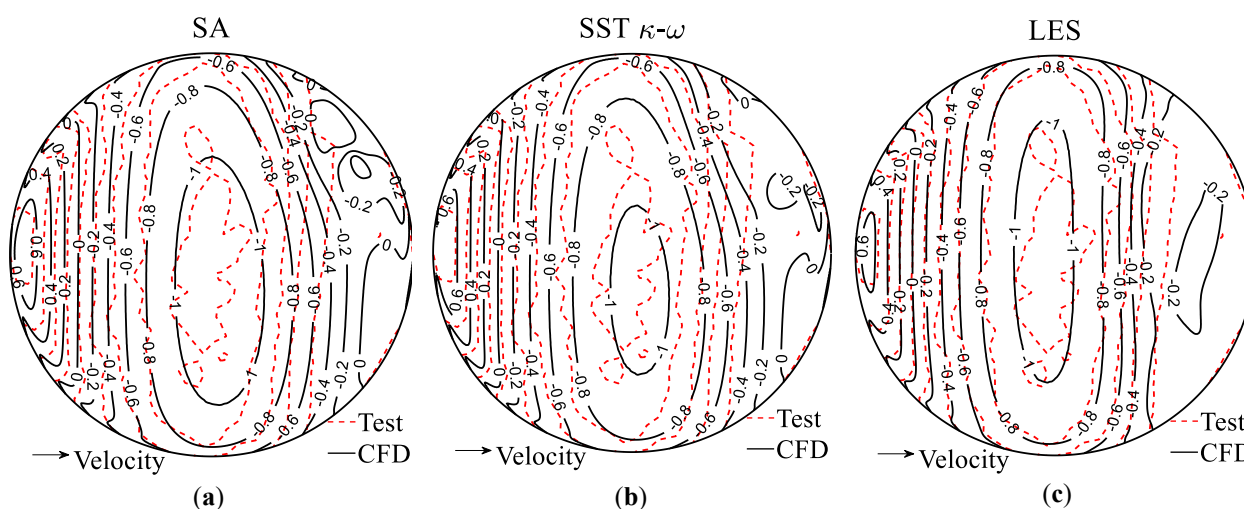


Figure 22. Comparisons of wind pressure distribution between numerical simulations and experimental results: (a) SA; (b) SST $k-\omega$; (c) LES.

Table 4. Comparisons of forces and moments between numerical and experimental results.

	Testing	SA	Error (%)	SST $k-\omega$	Error (%)	LES	Error (%)
F_x (N)	13633	12237	9.9	11443	16.1	14522	6.6
F_z (N)	24518	21920	1.9	20650	4.0	24280	1.1
M_y (N·m)	-57	-51.1	10.4	-47.2	17.2	-60.0	5.2
M_z (N·m)	-57	-51.1	10.4	-47.2	17.2	-60.0	5.2

4.2.2. Time Averaged Streamlines

Time averaged streamlines on the vertical $X-Z$ plane where $Y \equiv 0$, on the horizontal $X-Y$ plane where $Z \equiv 0.01$ m near the ground ($Z/D \equiv 1/200$), and on the horizontal $X-Y$ plane where $Z \equiv 0.167$ m ($Z/D \equiv 1/12$) are presented in Figures 23–25. From Figure 23, the airflow on the windward side splits at the stagnation point, and a horseshoe vortex forms in front of the dome due to the collision between the airflow below the stagnation point and the dome. Additionally, the horseshoe wraps around the dome and extends to the downstream region near the ground as observed in Figure 24. Taking Figure 22 into account, the occurrence of the maximum pressure on the windward side is related to the stagnation point, the occurrence of the maximum pressure on the windward side is related to the area and divergence of streamlines. Above the stagnation point, the blockage of the dome model causes the convergence of streamlines, and then the airflow accelerates around the dome apex. Meanwhile, negative pressure occurs and reaches its peak value at the apex of the dome (Figure 22). This can be explained by the mass continuity and Bernoulli theorems. On the leeward side, boundary layer separation and another recirculation region behind the dome are formed, and then the airflow decelerates. The associated area is affected by negative pressure. The above findings are consistent with the observations reported by [26,41].

ness. Figure 21 shows that boundary layer separations are reproduced for all numerical simulations. However, only the LES exhibits the two opposite rotating vortices. Even at the upper plane (Figure 25), rotating vortices are still invisible for the RANS simulations, while these vortices become stronger and extend more in the wake for the LES. In summary, the simulated time averaged streamlines in Figures 23–25 associated with the LES match better with other reported studies [26,27,41], and its back calculated point of boundary layer separation is closest to that estimated from the wind tunnel testing.

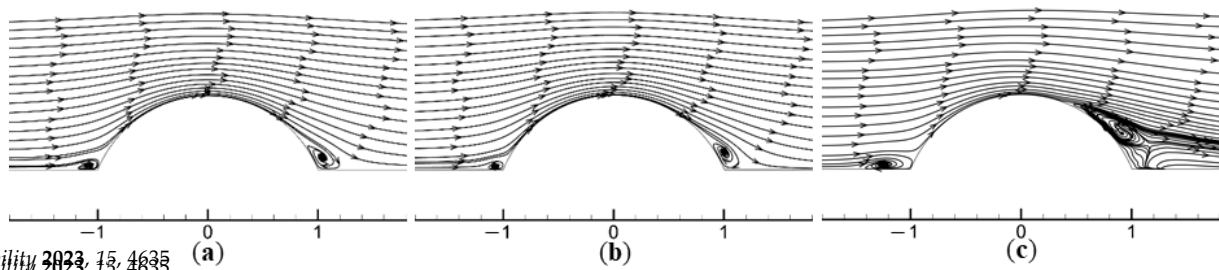


Figure 23. Streamlines on the vertical $X-Z$ plane where $Y = 0$: (a) SA; (b) SST $k-\omega$; (c) LES.

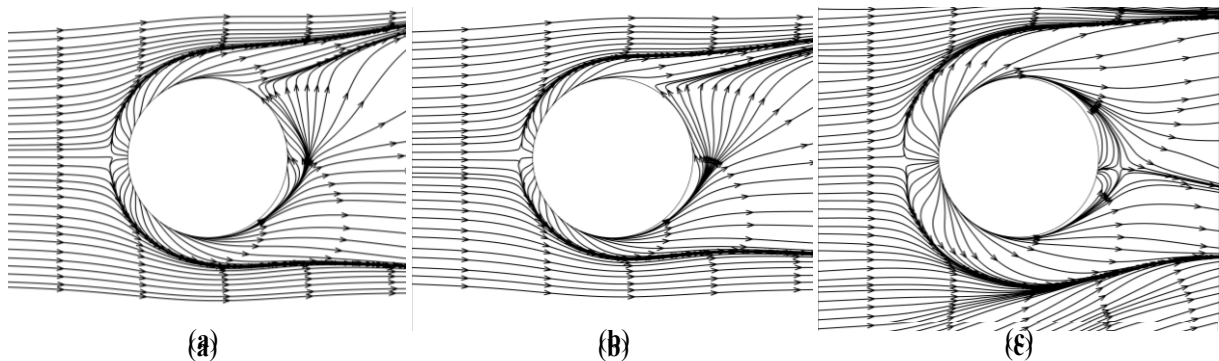


Figure 24. Streamlines on the horizontal $X-Y$ plane where $Z = 0.01$ m ($Z/D = 1/200$): (a) SA; (b) SST $k-\omega$; (c) LES.

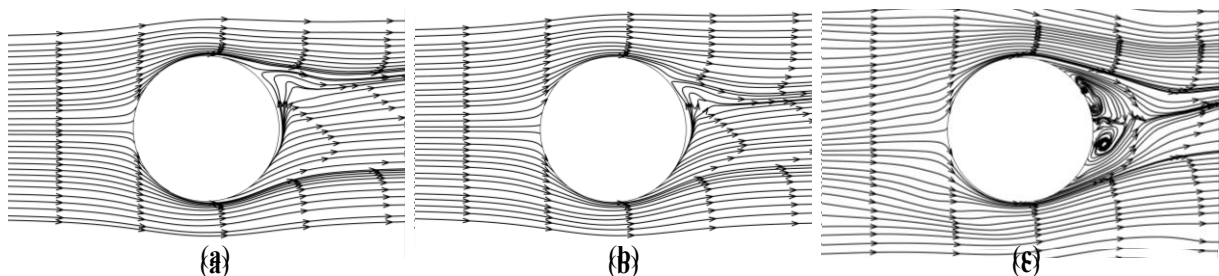
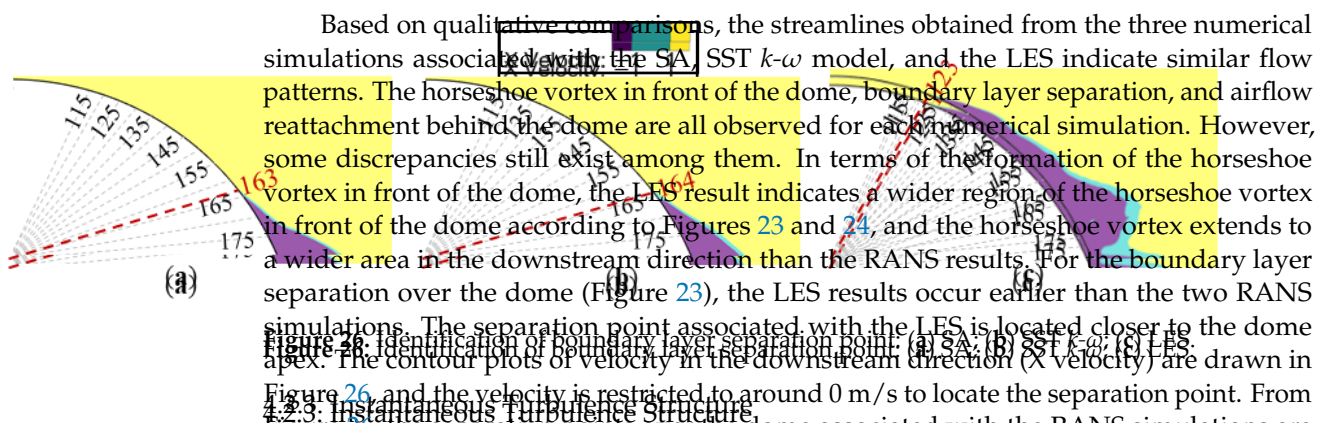


Figure 25. Streamlines on the horizontal $X-Y$ plane where $Z = 0.167$ m ($Z/D = 1/12$): (a) SA; (b) SST $k-\omega$; (c) LES.

Based on qualitative comparisons, the streamlines obtained from the three numerical simulations associated with the SA, SST $k-\omega$ model, and the LES indicate similar flow patterns. The horseshoe vortex in front of the dome, boundary layer separation, and airflow reattachment behind the dome are all observed for each numerical simulation. However, some discrepancies still exist among them. In terms of the formation of the horseshoe vortex in front of the dome, the LES result indicates a wider region of the horseshoe vortex in front of the dome according to Figures 23 and 24, and the horseshoe vortex extends to a wider area in the downstream direction than the RANS results. For the boundary layer separation over the dome (Figure 23), the LES results occur earlier than the two RANS simulations.

The separation point associated with the LES is located closer to the dome apex. Figure 26: Identification of boundary layer separation point: (a) SA; (b) SST $k-\omega$; (c) LES.



The contour plots of velocity in the downstream direction (X velocity) are drawn in Figure 26, and the velocity is restricted to around 0 m/s to locate the separation point. From Figure 26, the separation points over the dome associated with the RANS simulations are located at 165–164 degrees, while the separation point determined from the LES is located at 123 degrees (Figure 27).

The instantaneous turbulence structures around the dome associated with the LES and SST $k-\omega$ model are depicted in Figure 27 based on the Q criterion [42]. Q is derived as the second invariant of the velocity gradient tensor, which defines vortices as areas where the vorticity magnitude is greater than the magnitude of strain. Positive values of Q indicate the existence of a vortex where vorticity dominates. The iso surface of the Q criterion presented in Figure 27 is colored by simulation pressure coefficients. From Figure 27a, on the windward side of the dome, the boundary layer vorticity forms because of the ground friction. Then, the accumulation of the boundary layer vorticity and the collision between the flow below the stagnation point and the dome leads to the appearance of horseshoe vortices, which wrap around the dome and contribute to the trailing vorticity in the wake region. On the leeward side of the dome, boundary layer separation occurs

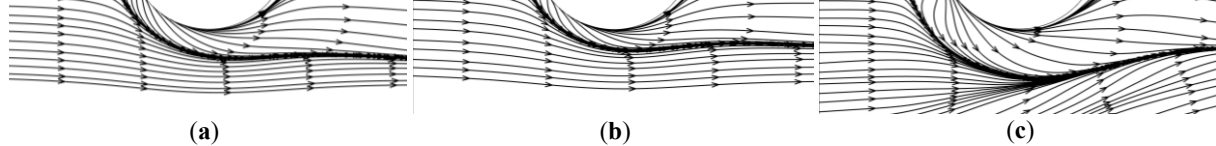


Figure 24. Streamlines on the horizontal X-Y plane where $Z = 0.01$ m ($Z/D = 1/200$): (a) SA; (b) SST $k-\omega$; (c) LES.

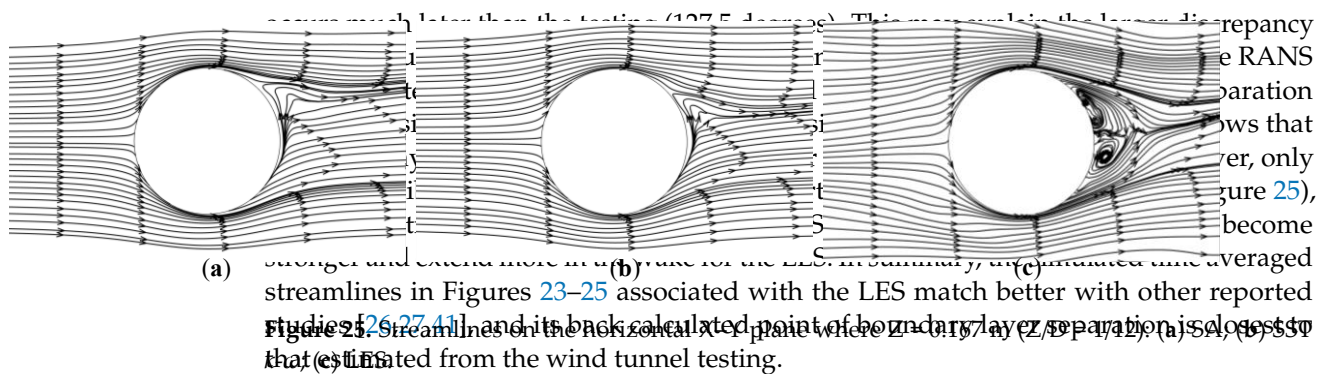


Figure 25. Streamlines on the horizontal X-Y plane where $Z = 0.17$ m ($Z/D = 1/12$): (a) SA; (b) SST $k-\omega$; (c) LES.

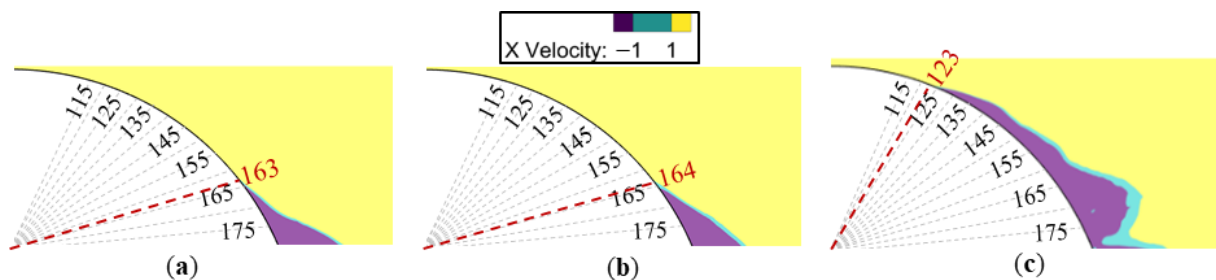


Figure 26. Identification of boundary layer separation point: (a) SA; (b) SST $k-\omega$; (c) LES.

4.2.3. Instantaneous Turbulence Structure

The instantaneous turbulence structures around the dome associated with the LES and SST $k-\omega$ model are depicted in Figure 27 based on the Q criterion [42]. Q is derived as the second invariant of the velocity gradient tensor, which defines vortices as areas where the vorticity magnitude is greater than the magnitude of the rate of strain. Positive values of Q indicate the existence of a vortex where vorticity dominates. The iso-surface of the Q criterion presented in Figure 27 is colored by the mean pressure coefficients. From Figure 27a, on the windward side of the dome, the boundary layer vorticity forms because of the ground friction. Then, the accumulation of the boundary layer vorticity and the collision between the flow below the stagnation point and the dome leads to the appearance of horseshoe vortices, which wrap around the dome and contribute to the trailing vorticity in the wake region. On the leeward side of the dome, boundary layer separation occurs and causes the formation of small-scale vortices, which then merge with each other and develop into larger-scale strip vortices and finally disappear in the far wake region. The presented turbulence structure is similar to the experimental observation of a dome with a Reynolds number of 1.6×10^5 reported by [41]. Figure 27b depicts the turbulence structure associated with the SST $k-\omega$ model, and it is clear that the RANS simulations cannot provide such unsteady information.

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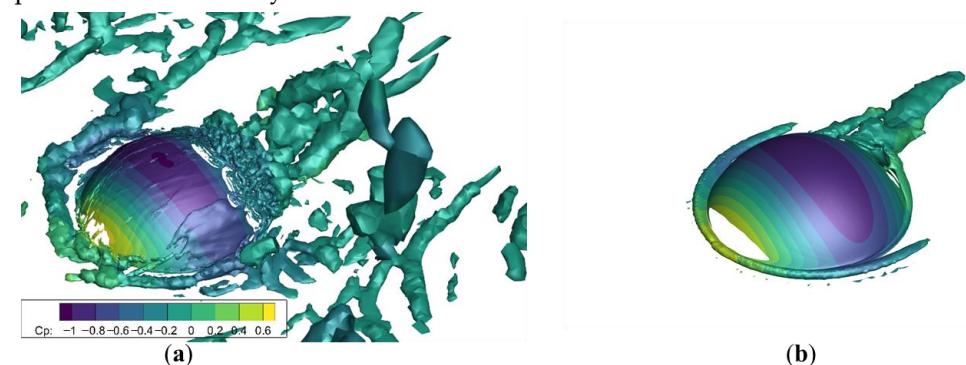


Figure 27. Iso-surfaces of Q criterion ($Q = 3000$) colored by mean pressure coefficients: (a) LES; (b) SST $k-\omega$.

5. Conclusions

In the study, wind effects acting on dome structures are investigated based on scale (1/60) wind tunnel testing and CFD simulations. In the wind tunnel testing

5. Conclusions

In the study, wind effects acting on dome structures are investigated based on large-scale (1/60) wind tunnel testing and CFD simulations. In the wind tunnel testing, both open and suburban terrain configurations are considered, and three velocity levels are applied to the dome model. Based on the experimental results, the following summary can be made. The magnitude of the mean pressure coefficient generally increases with the inflow wind velocity and hence, increasing wind forces, and the magnitude of the pressure coefficient for the suburban terrain is lower than that for the open terrain. The maximum pressure coefficient occurs at an elevation of about 20 degrees, and the minimum pressure coefficient appears at around 90 degrees. Suction force is the most significant among all wind-induced loads, which may damage the roof or even lift the roof up, and therefore, special attention should be paid to wind resistance design.

The LES and RANS simulations with the Spalart–Allmaras model and SST $k-\omega$ model are performed and their performances are validated against the wind tunnel testing. The following conclusions can be drawn based on the numerical results. In terms of the mean pressure coefficients on the center meridian and the overall wind pressure distribution on the dome surface, the mean and root-mean-square errors associated with the LES are much smaller than the RANS. Regarding time averaged flow patterns, although both the RANS and LES can capture the horseshoe vortex, boundary layer separation, and reattachment behind the dome, the LES is able to predict the location of the boundary layer separation more accurately and regenerate stronger and more detailed vortices in the wake region. Moreover, more details of the instantaneous turbulence structure around the dome model can be provided by the LES, while this cannot be achieved by the RANS simulations.

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References

1. Maher, F.J. Wind loads on basic dome shapes. *J. Struct. Div.* **1965**, *91*, 219–228. [[CrossRef](#)]
2. Taniguchi, S.; Sakamoto, H.; Kiya, M.; Arie, M. Time-averaged aerodynamic forces acting on a hemisphere immersed in a turbulent boundary. *J. Wind Eng. Ind. Aerod.* **1982**, *9*, 257–273. [[CrossRef](#)]
3. Taylor, T.J. Wind pressures on a hemispherical dome. *J. Wind Eng. Ind. Aerod.* **1992**, *40*, 199–213. [[CrossRef](#)]
4. Ogawa, T.; Nakayama, M.; Murayama, S.; Sasaki, Y. Characteristics of wind pressures on basic structures with curved surfaces and their response in turbulent flow. *J. Wind Eng. Ind. Aerod.* **1991**, *38*, 427–438. [[CrossRef](#)]
5. Savory, E.; Toy, N. Hemisphere and hemisphere-cylinders in turbulent boundary layers. *J. Wind Eng. Ind. Aerod.* **1986**, *23*, 345–364. [[CrossRef](#)]
6. Letchford, C.W.; Sarkar, P.P. Mean and fluctuating wind loads on rough and smooth parabolic domes. *J. Wind Eng. Ind. Aerod.* **2000**, *88*, 101–117. [[CrossRef](#)]

7. Cheng, C.M.; Fu, C.L. Characteristic of wind loads on a hemispherical dome in smooth flow and turbulent boundary layer flow. *J. Wind Eng. Ind. Aerod.* **2010**, *98*, 328–344. [\[CrossRef\]](#)
8. Chevula, S.; Sanz-Andres, A.; Franchini, S. Aerodynamic external pressure loads on a semi-circular bluff body under wind gusts. *J. Fluid Struct.* **2015**, *54*, 947–957. [\[CrossRef\]](#)
9. Kim, Y.C.; Yoon, S.W.; Cheon, D.J.; Song, J.Y. Characteristics of wind pressures on retractable dome roofs and external peak pressure coefficients for cladding design. *J. Wind Eng. Ind. Aerod.* **2019**, *188*, 294–307. [\[CrossRef\]](#)
10. Lee, J.H.; Kim, Y.C.; Cheon, D.J.; Yoon, S.W. Wind pressure characteristics of elliptical retractable dome roofs. *J. Asian Archit. Build.* **2022**, *21*, 1561–1577. [\[CrossRef\]](#)
11. Tavakol, M.M.; Yaghoubi, M.; Ahmadi, G. Experimental and numerical analysis of airflow around a building model with an array of domes. *J. Build. Eng.* **2021**, *34*, 101901. [\[CrossRef\]](#)
12. Murakami, S. Overview of turbulence models applied in CWE-1997. *J. Wind Eng. Ind. Aerod.* **1998**, *74–76*, 1–24. [\[CrossRef\]](#)
13. Smagorinsky, J. General circulation experiments with the primitive equations: I. The basic experiment. *Mon. Weather Rev.* **1963**, *91*, 99–164. [\[CrossRef\]](#)
14. Nicoud, F.; Ducros, F. Subgrid-scale stress modelling based on the square of the velocity gradient tensor. *Flow Turbul. Combust.* **1999**, *62*, 183–200. [\[CrossRef\]](#)
15. Germano, M.; Piomelli, U.; Moin, P.; Cabot, W.H. A dynamic subgrid-scale eddy viscosity model. *Phys. Fluids A-Fluid* **1991**, *3*, 1760. [\[CrossRef\]](#)
16. Lilly, D.K. A proposed modification of the Germano subgrid-scale closure method. *Phys. Fluids A-Fluid* **1992**, *4*, 633–635. [\[CrossRef\]](#)
17. Spalart, P.; Allmaras, S. A one-equation turbulence model for aerodynamic flows. *Rech. Aerospaciale* **1994**, *1*, 5–21. [\[CrossRef\]](#)
18. Launder, B.E.; Spalding, D.B. *Lectures in Mathematical Models of Turbulence*; Academic Press: London, UK, 1972.
19. Wilcox, D.C. *Turbulence Modeling for CFD*; DCW Industries: La Canada Flintridge, CA, USA, 1998.
20. Shih, T.H.; Liou, W.W.; Shabbir, A.; Yang, Z.; Zhu, J. A new k- ϵ eddy viscosity model for high reynolds number turbulent flows. *Comput. Fluids* **1995**, *24*, 227–338. [\[CrossRef\]](#)
21. Yakhot, V.S.; Orszag, S.A.; Thangam, S.; Gatski, T.B.; Speziale, C.G. Development of turbulence models for shear flows by a double expansion technique. *Phys. Fluids A-Fluid* **1992**, *4*, 1510. [\[CrossRef\]](#)
22. Menter, F.R. Two-equation eddy-viscosity turbulence models for engineering applications. *AIAA J.* **1994**, *32*, 1598–1605. [\[CrossRef\]](#)
23. Menter, F.R. Eddy viscosity transport equations and their relation to the k- ϵ model. *J. Fluid Eng.* **1997**, *119*, 876–884. [\[CrossRef\]](#)
24. Menter, F.R.; Kuntz, M.; Langtry, R. Ten years of industrial experience with the SST turbulence model. *Turbul. Heat Mass Transf.* **2003**, *4*, 625–632.
25. Tavakol, M.M.; Yaghoubi, M.; Motlagh, M.M. Air flow aerodynamic on a wall-mounted hemisphere for various turbulent boundary layers. *Exp. Therm. Fluid Sci.* **2010**, *34*, 538–553. [\[CrossRef\]](#)
26. Tavakol, M.M.; Abouali, O.; Yaghoubi, M. Large eddy simulation of turbulent flow around a wall mounted hemisphere. *Appl. Math Model* **2015**, *39*, 3596–3618. [\[CrossRef\]](#)
27. Fu, C.L.; Cheng, C.M.; Lo, Y.L.; Cheng, D.Q. LES simulation of hemispherical dome's aerodynamic characteristics in smooth and turbulence boundary layer flows. *J. Wind Eng. Ind. Aerod.* **2015**, *144*, 53–61. [\[CrossRef\]](#)
28. Wood, J.N.; De Nayer, G.; Schmidt, S.; Breuer, M. Experimental investigation and large-eddy simulation of the turbulent flow past a smooth and rigid hemisphere. *Flow Turbul. Combust.* **2016**, *97*, 79–119. [\[CrossRef\]](#)
29. Sadeghi, H.; Heristchian, M.; Aziminejad, A.; Nooshin, H. Wind effect on grooved and scallop domes. *Eng. Struct.* **2017**, *148*, 436–450. [\[CrossRef\]](#)
30. Fouad, N.S.; Mahmoud, G.H.; Nasr, N.E. Comparative study of international codes wind loads and CFD results for low rise buildings. *Alex. Eng. J.* **2018**, *57*, 3623–3639. [\[CrossRef\]](#)
31. Khosrowjerdi, S.; Sarkardeh, H.; Kioumars, M. Effect of wind load on different heritage dome buildings. *Eur. Phys. J. Plus* **2021**, *136*, 1180. [\[CrossRef\]](#)
32. Franke, J.; Hirsch, C.; Jensen, A.G.; Krüs, H.W.; Schatzmann, M.; Westbury, P.S.; Miles, S.D.; Wisse, J.A.; Wright, N.G. Recommendations on the use of CFD in predicting pedestrian wind environment. In *COST Action C14 "Impact of Wind and Storms on City Life and Built Environment"*; Von Karman Institute: Rhode-Saint-Genese, Belgium, 2004.
33. Murakami, S. Setting the scene: CFD and symposium overview. *Wind Struct.* **2002**, *5*, 83–88. [\[CrossRef\]](#)
34. Tominaga, Y.; Stathopoulos, T. Numerical simulation of dispersion around an isolated cubic building: Model evaluation of RANS and LES. *Build. Environ.* **2010**, *45*, 2231–2239. [\[CrossRef\]](#)
35. American Society of Civil Engineers. *Minimum Design Loads and Associated Criteria for Buildings and Other Structures*; ASCE/SEI 7-16; American Society of Civil Engineers: Reston, VA, USA, 2016. [\[CrossRef\]](#)
36. Simiu, E.; Scanlan, R.H. *Wind Effects on Structures: Fundamentals and Applications to Design*, 3rd ed.; John Wiley and Sons, Inc.: New York, NY, USA, 1986.
37. Kaimal, J.C.; Finnigan, J.J. *Atmospheric Boundary Layer Flows: Their Structure and Measurement*; Oxford University Press: Oxford, UK, 1994.
38. Pope, S.B. *Turbulent Flows*; Cambridge University Press: Cambridge, UK, 2000; pp. 182–263.
39. Blocken, B. Computational Fluid Dynamics for urban physics: Importance, scales, possibilities, limitations and ten tips and tricks towards accurate and reliable simulations. *Build. Environ.* **2015**, *91*, 219–245. [\[CrossRef\]](#)

40. Franke, J.; Hellsten, A.; Schlünzen, H.; Carissimo, B. Best practice guideline for the CFD simulation of flows in the urban environment. In Proceedings of the 11th Conference on Harmonisation within Atmospheric Dispersion Modelling for Regulatory Purposes, Cambridge, UK, 2–5 July 2007.
41. Savory, E.; Toy, N. The separated shear layers associated with hemispherical bodies in turbulent boundary layers. *J. Wind Eng. Ind. Aerod.* **1988**, *28*, 291–300. [[CrossRef](#)]
42. Hunt, J.C.; Wray, A.A.; Moin, P. Eddies, streams, and convergence zones in turbulent flows. In Proceedings of the 1988 Summer Program. Center for Turbulence Research, Buffalo, NY, USA, 20–22 November 1988.

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